Appendix K: Traffic and Parking Analysis

K.1 - MARINA PARK TPO TRAFFIC ANALYSIS

MARINA PARK TPO

Traffic Analysis

September 2009



MARINA PARK TPO TRAFFIC ANALYSIS

Prepared by:

Austin-Foust Associates, Inc. 2020 North Tustin Avenue Santa Ana, California 92705-7827 (714) 667-0496 MARINA PARK TPO TRAFFIC ANALYSIS

This report summarizes an analysis performed for the proposed development of Marina Park in

Newport Beach based on the City's Traffic Phasing Ordinance (TPO) methodology. In addition, this

report summarizes the results of an analysis of cumulative conditions in compliance with California

Environmental Quality Act (CEQA) requirements.

PROJECT DESCRIPTION

The proposed Marina Park project is located on the Bay side of Balboa Boulevard between 18th

Street and 16th Street on the Balboa Peninsula in the City of Newport Beach. The approximately nine-

acre site is currently developed with a 57-unit mobile home park, a community center, four tennis courts,

a small playground, and the Girl Scout House. Adjacent to the project site between 16th Street and 15th

Street is the existing American Legion Post 291, a small residential development, and a commercial

building. Figure 1 illustrates the location of the proposed Marina Park project.

Phase 1 of the proposed Marina Park consists of removing the existing mobile homes (57 DU) and

replacing them with 3.83 acres of sand. Approximately 112 metered parking spaces will be provided.

Other than temporary restroom facilities, no other facilities will be provided. The existing community

center, Girl Scout House, tennis courts, and small playground will remain. Figure 2 illustrates the

proposed Phase 1 concept plan.

Phase 2 of the proposed Marina Park consists of replacing the 3.83 acres of sand in Phase 1 with

3.83 acres of turf. There will be no change in the amount of metered parking provided. Other than

temporary restroom facilities, no other facilities will be provided. The existing community center, Girl

Scout House, tennis courts, and small playground will remain. Figure 3 illustrates the proposed Phase 2

concept plan.

Phase 3 of the proposed Marina Park consists of a 10,200 square foot Community Center, an

11,200 square foot Sailing Center, recreational park uses, and a 23-berth non-commercial Visitor Marina.

The Sailing Center will include a 54-seat café. A playground, two tennis courts, two half-court basketball

courts, beach volleyball courts, and open lawn areas for picnicking and free play are proposed. New

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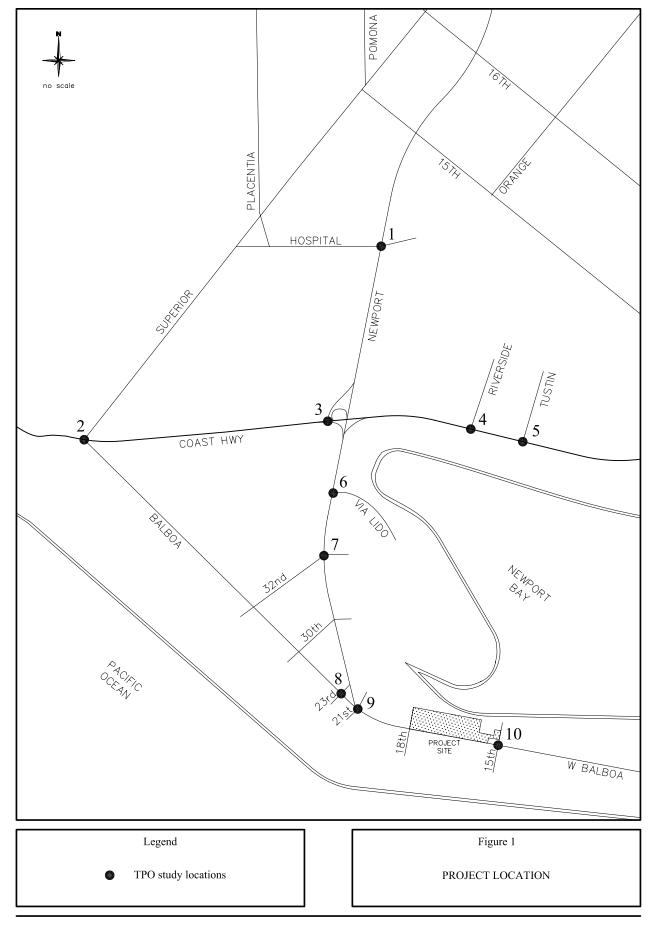


Figure 2
PROPOSED CONCEPT PLAN
- PHASE 1

Marina Park TPO Traffic Analysis

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Figure 3
PROPOSED CONCEPT PLAN
- PHASE 2

Marina Park TPO Traffic Analysis

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docks and slips for sailing programs and expanded and improved beach access will be provided. The existing Girl Scout House will be relocated to the northwest corner of the project site. Figure 4 illustrates the proposed Phase 3 Marina Park concept plan.

TRIP GENERATION AND DISTRIBUTION

Phase 1 and Phase 2

Phase 1 will expand the lightly used existing sand area on the Bay side beach between 18th Street and 16th Street by 3.83 acres for a total of 5.99 acres. Phase 2 will simply replace the 3.83 acres of sand with 3.83 acres of turf. Neither Phase 1 nor Phase 2 will provide amenities such as sports facilities or playground equipment.

Trip rates for Phase 1 and Phase 2 of the proposed project were derived from Institute of Transportation Engineers (ITE) Trip Generation, Eighth Edition trip rates. The ITE Beach Park peak hour and daily rates per acre were applied to Phase 1 and Phase 2 of the proposed project. Credit for the trips currently being generated by the existing mobile home park were determined from ITE trip rates.

Table 1 summarizes the resulting trip generation for Phase 1 and Phase 2 of the proposed project. As this table shows, Phase 1 and Phase 2 result in a decrease in trips when compared with the trips generated by the existing mobile homes on the site. Since the existing mobile homes on the site generate more peak hour and daily trips than Phase 1 and Phase 2 of the proposed project, Phase 1 and Phase 2 will have no significant impact on the study area and no additional analysis of Phase 1 and Phase 2 is required.

Phase 3

Phase 3 of the proposed project consists of a 10,200 square foot Community Center, an 11,200 square foot Sailing Center, recreational park uses, and a 23-berth non-commercial Visitor Marina. The proposed Community Center will include three ground-floor classrooms for use by the sailing program or other City programs, and the second floor will provide administrative functions and a large room that accommodates up to 80 people for use as a large classroom, four small classrooms, or a banquet facility.

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Figure 4
PROPOSED CONCEPT PLAN
- PHASE 3

Table 1 TRIP GENERATION SUMMARY – PHASE 1 & PHASE 2 AM PEAK HOUR PM PEAK HOUR UNITS LAND USE IN OUT TOTALIN OUT TOTAL ADT TRIP RATES Beach Park (ITE 415) Acre .28 .20 .48 .38 .92 1.30 29.81 Mobile Home Park (ITE 240) DU .09 .35 .44 .37 .22 .59 4.99 TRIP GENERATION Proposed Phase 1 & Phase 2 179 5.99 Acres 2 1 3 2 Park 6 8 **Existing Use** Mobile Home Park 57 DU -5 -20 -25 -21 -13 -34 -285 **NET NEW TRIPS** -106 -3 -19 -22 -19 -7 -26

The Sailing Center consists of a reception area, a medium sized meeting room, and a two-story gallery space. The second floor will include a 54-seat café and administrative offices. The Sailing Center will provide indoor space for storage and maintenance of the boats and equipment used in the sailing program.

The Visitor Marina includes 21 40-foot long berths and two 50-foot long berths for use by the public for short-term visiting non-commercial vessels plus a 200 foot long dock. Full hook-ups will be provided to all berths. The marina will be fenced and controlled security access provided at the southeastern corner. A marina control building containing an office, restrooms, and washing machines will be provided adjacent to the marina entry point.

Amenities in the park will include picnic tables, restrooms, showers, play areas, half-court basketball courts, benches, public beach and access to the water. Tennis courts are located adjacent to the American Legion.

The entire site is currently developed with a 57-unit mobile home park, community center, Girl Scout House, tennis courts, and playground which are generating peak hour and daily traffic. Credit for the trips currently being generated by the existing mobile home park, community center, tennis courts and playground were determined from ITE trip rates.

The Girl Scout House will be relocated from its current location to the northwest corner of the site. No changes in the trips generated by the Girl Scout House are expected.

Trip generation rates for Phase 3 of the proposed project were derived from peak hour and daily trip rates contained in ITE Trip Generation, Eighth Edition. A combined rate that consists of the ITE Beach Park AM and PM peak hour rates per acre and an average of the ITE City Park and Beach Park daily rates per acre was applied to the park uses. ITE's Recreational Community Center trip rates were applied to the proposed Community Center and Sailing Center, which includes a 54-seat café. These rates and the resulting trips for Phase 3 of the project are summarized in Table 2.

The existing trips from the mobile home park, the tennis courts, and the playground were subtracted from the proposed trip generation to produce the net new trips for the project. As the trip generation table indicates, the proposed project results in a net increase of 261 trips daily. During the AM

Table 2 TRIP GENERATION SUMMARY - PHASE 3

		AN	I PEAK I	HOUR	PM	1 PEAK I	HOUR	
LAND USE	UNITS	IN	OUT	TOTAL	IN	OUT	TOTAL	ADT
TRIP RATES								
Park ¹	Acre	.28	.20	.48	.38	.92	1.30	15.70
Recreational Community Center (ITE	495) ² TSF	.99	.63	1.62	.48	1.16	1.64	22.88
Marina (ITE 420)	Berth	.03	.05	.08	.11	.08	.19	2.96
Mobile Home Park (ITE 240)	DU	.09	.35	.44	.37	.22	.59	4.99
TRIP GENERATION								
Proposed Project								
Park	4.89 Acres	1	1	2	2	4	6	77
Community Ctr/Sailing Ctr/Cafe	21.3 TSF	21	13	34	10	25	35	487
Visitor Marina	23 Berths	1	1	2	3	2	5	68
Sub-Total		23	15	38	15	31	46	632
Existing Use								
Mobile Home Park	57 DU	-5	-20	-25	-21	-13	-34	-285
Park	1.2 Acres	0	0	0	0	-1	-1	-19
Community Ctr	2.9 TSF	-3	-2	-5	-1	-4	-5	-67
NET NEW TRIPS		15	-7	8	-7	13	6	261

Notes:

The Girl Scout House will be relocated on-site and results in no net change in project trips.

¹ Park AM and PM trip rates from ITE Beach Park (415) rate/acre and ADT rate averaged from City (411) and Beach (415) Park ADT rate/acre.

² ITE Recreational Community Center (495) trip rates applied to Community Center, Sailing Center, and Café.

peak hour there is a net increase of eight trips generated by Phase 3 of the project compared with the trip generation of the existing uses on the site. During the PM peak hour Phase 3 of the proposed project generates six new trips compared with the trip generation of the existing uses on the site.

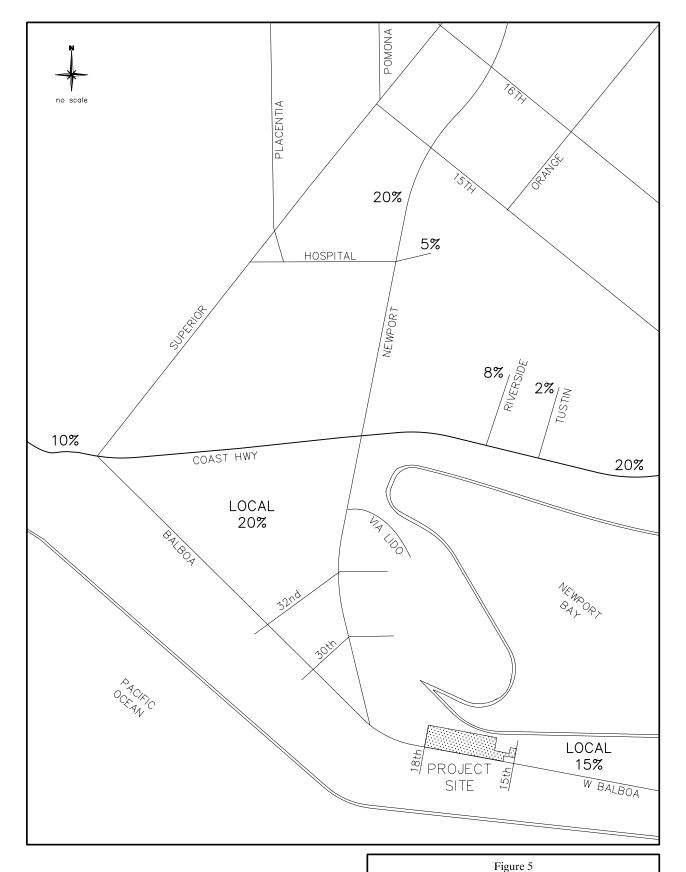
Trip distribution of Phase 3 project-generated traffic onto the surrounding circulation system was determined from observed travel patterns in the vicinity of the project site as well as from locations and levels of development in relation to the subject property. A large portion of trips generated by Phase 3 of the project are estimated to originate within the City of Newport Beach. Approximately 35 percent of Phase 3 project trips are oriented toward the areas south of Coast Highway, including the Balboa Peninsula area. The remaining 65 percent of Phase 3 project traffic is distributed along Coast Highway and Newport Boulevard. The general distribution for Phase 3 of the proposed development is illustrated in Figure 5. Phase 3 project-generated trips were distributed to the circulation system according to these distribution patterns. The AM and PM peak hour trips for Phase 3 of the proposed development are illustrated in Appendix A.

TPO TRAFFIC IMPACTS

The City of Newport Beach identified ten intersections for analysis to determine the impact of the proposed Marina Park development. These intersections are:

Newport Boulevard and Hospital Road
Balboa Boulevard/Superior Avenue and Coast Highway
Newport Boulevard and Coast Highway
Riverside Avenue and Coast Highway
Tustin Avenue and Coast Highway
Newport Boulevard and Via Lido
Newport Boulevard and 32nd Street
23rd Street and Balboa Boulevard
21st Street and Balboa Boulevard

Existing peak hour intersection volumes for the study locations were provided by City Staff (existing peak hour volumes are illustrated in Appendix A) with the exceptions of 23rd Street at Balboa Boulevard, 21st Street at Balboa Boulevard, and 15th Street at Balboa Boulevard, which were counted by



GENERAL PROJECT PHASE 3 DISTRIBUTION

Traffic Data Services, Inc. in September 2009. The peak hour data provided by the City was collected during the non-peak season in 2006, 2007, and 2008. Existing intersection levels of service are based on intersection capacity utilization (ICU) values. The ICU values are a means of presenting the volume to capacity ratios, with a V/C ratio of .90 representing the upper threshold for an acceptable level of service (LOS "D") in the City of Newport Beach. The City methodology calculates the ICU value to three decimal places, and then reports the resulting ICU value rounded to two decimal places.

Existing ICU values for the study intersections assuming existing lane configurations are summarized in Table 3 (actual ICU calculation sheets are included in Appendix B). As this table shows, the study intersections are currently operating at LOS "D" or better during the AM and PM peak hours. These ICU values represent the non-peak season conditions.

Sample mid-block counts were collected to determine the seasonal increase in Summer traffic volumes for this area of Newport Beach. Counts collected on Newport Boulevard at 32nd Street and on Balboa Boulevard at 18th Street in early June 2008 (non-peak season) and late June 2008 (peak Summer season) indicate an average increase of 18 percent in the daily traffic volume during the Summer (count data is included in Appendix D). During the peak hours, the Summer increase averages 17 percent over the non-peak season volume during the AM peak hour and 16 percent during the PM peak hour. Existing peak hour volumes at the study intersections were increased to Summer conditions (illustrated in Appendix A), and the resulting Summer season ICU values are summarized in Table 4.

An ambient growth rate of 1.0 percent per year was added to the existing volumes along Newport Boulevard north of Coast Highway and along Coast Highway. Construction of Phase 3 of the project is assumed to be complete in 2010; therefore, the study year is 2011. Traffic generated by approved projects in the study area, obtained from City Staff, were added to the existing peak hour volumes to obtain year 2011 background peak hour volumes for the intersections prior to the addition of project-generated traffic (illustrated in Appendix A). Table 5 summarizes the approved projects included in this analysis.

Background-plus-project Phase 3 peak hour volumes were obtained by adding the Phase 3 project-generated peak hour intersection volumes presented above to the existing-plus-regional growth-plus-approved projects peak hour volumes. Background-plus-project Phase 3 peak hour volumes are illustrated in Appendix A.

Table 3								
EXISTING ICU ANALYSIS SUMMARY – NON-PEAK SEASON								
INTERSECTION	AM	PM						
 Newport & Hospital Balboa/Superior & Coast Hyw Newport & Coast Hwy Riverside & Coast Hwy Tustin & Coast Hwy Newport & Via Lido Newport & 32nd 23rd & Balboa 21st & Balboa Balboa & 15th 	.59 .68 .77 .70 .67 .47 .48 .11 .23	.64 .72 .68 .81 .58 .43 .66 .12 .32						
Level of service ranges: .0060 A .6170 B .7180 C .8190 D .91 - 1.00 E Above 1.00 F								

Table 4								
EXISTING ICU ANALYSIS SUMMARY – SUMMER SEASON								
INTERSECTION	AM	PM						
 Newport & Hospital Balboa/Superior & Coast Hyw Newport & Coast Hwy Riverside & Coast Hwy Tustin & Coast Hwy Newport & Via Lido Newport & 32nd 23rd & Balboa 21st & Balboa Balboa & 15th 	.66 .79 .90 .80 .78 .54 .56 .13 .28	.70 .82 .77 .88 .65 .49 .74 .14						
Level of service ranges: .0060 A .6170 B .7180 C .8190 D .91 - 1.00 E Above 1.00 F								

Table 5								
APPROVED PROJECTS SUMMARY								
LOCATION	PERCENT COMPLETE							
Fashion Island Expansion	40							
Temple Bat Yahm Expansion	65							
CIOSA – Irvine Project	91							
Newport Dunes	0							
1401 Dove Street	0							
1901 Westcliff Surgical Center	0							
Hoag Hospital Phase III	0							
Birch Medical Office	0							
St Mark Presbyterian Church	77							
Corporate Plaza West	0							
Mariner's Mile Gateway	0							
Land Rover NB Service Center	0							
Our Lady Queen of Angels Church Expansion	0							
2300 Newport Boulevard	0							
Newport Executive Court	0							
Hoag Health Center	0							
North Newport Center	0							
Santa Barbara Condo	0							

The TPO analysis consists of a one percent analysis and an ICU analysis at each study intersection. The one percent analysis compares the proposed Phase 3 project traffic with projected background peak hour volumes. To pass the one percent analysis, peak hour traffic from Phase 3 of the proposed project must be less than one percent of the projected background peak hour traffic on each leg of the intersection. If Phase 3 of the proposed project passes the one percent analysis, then the ICU analysis is not required and no further analysis is necessary. If Phase 3 of the proposed project does not pass the one percent analysis, then the ICU analysis must be performed for the intersection which fails to pass the one percent test.

Table 6 summarizes the results of the non-peak season one percent analysis for Phase 3 of the project (the one percent analysis sheets are included in Appendix C). As this table indicates, Phase 3 of the proposed project does not pass the one percent analysis at two study intersections during the AM and PM peak hour; therefore, an ICU analysis is required for the intersections of 23rd Street at Balboa Boulevard and 21st Street at Balboa Boulevard. The non-peak season volumes represent the worst-case one percent analysis since the Summer season volumes increase the background level against which the project trips are compared. If Phase 3 of the project passes the one percent analysis at a location under non-peak season conditions, then Phase 3 of the project will pass the one percent analysis at that location under Summer season conditions.

An ICU analysis was performed for the two intersections which did not pass the one percent test. Existing lane configurations were assumed, and a capacity of 1,600 vph per lane with no clearance factor was utilized. Table 7 summarizes the existing, existing-plus-project Phase 3, background, and background-plus-project Phase 3 ICU values during the AM and PM peak hours under non-peak season and Summer season conditions (actual ICU calculation sheets are included in Appendix B).

As the ICU summary table indicates, Phase 3 of the project will have no marginal impact on the intersections of 23rd Street at Balboa Boulevard and 21st Street at Balboa Boulevard which will operate at LOS "A" during the AM and PM peak hours under non-peak season and Summer season conditions. Phase 3 of the project has no significant impact on the study intersections, and no mitigation is required.

CUMULATIVE CONDITIONS ANALYSIS

City Staff provided a list of 12 known but not approved projects for use in a cumulative conditions analysis. These cumulative projects are summarized in Table 8. Trip generation and distribution for each

 $\label{eq:table 6}$ SUMMARY OF PHASE 3 ONE PERCENT ANALYSIS – NON-PEAK SEASON

	DD C	AM PEAK		MEG	LEGGETTAN 10/ OF
7. TTTTT 07. CTT 0.1.		IECT PHAS	LESS THAN 1% OF		
INTERSECTION	NB	SB	EB	WB	PEAK HOUR VOLUMES
Newport & Hospital	0	3	0	1	Yes
2. Balboa/Superior & Coast Hwy	0	0	1	0	Yes
3. Newport & Coast Hwy	0	0	0	5	Yes
4. Riverside & Coast Hwy	0	1	0	3	Yes
5. Tustin & Coast Hwy	0	0	0	3	Yes
6. Newport & Via Lido	0	9	0	0	Yes
7. Newport & 32nd	0	9	1	0	Yes
8. Balboa & 23rd	0	0	3	0	No
9. Balboa & 21st	0	0	13	0	No
10. 15th & Balboa	0	0	0	2	Yes
		PM PEAK	HOUR		
	PRO1	IECT PHAS		ЛES	LESS THAN 1% OF
INTERSECTION	NB	SB	EB	WB	PEAK HOUR VOLUMES
 Newport & Hospital 	4	0	0	0	Yes
2. Balboa/Superior & Coast Hwy	1	0	0	0	Yes
3. Newport & Coast Hwy	0	0	0	0	Yes
4. Riverside & Coast Hwy	0	0	4	0	Yes
5. Tustin & Coast Hwy	0	0	3	0	Yes
6. Newport & Via Lido	7	0	0	0	Yes
7. Newport & 32nd	8	0	0	0	Yes
8. Balboa & 23rd	0	0	0	3	No
9. Balboa & 21st	0	0	0	11	No
10. 15th & Balboa	0	0	2	0	Yes

			Table 7					
ICU ANALYSIS SUMMARY – PHASE 3								
	EXISTING + BACKGROUND EXISTING PROJECT PHASE 3 BACKGROUND PROJECT PHAS							ROUND +
INTERSECTION	AM	PM	AM	PM	AM	PM	AM	PM
INTERSECTION	7 11/1	1 1/1	7 XIVI	1 1/1	1 11/1	1 1/1	7 XIVI	1 1/1
Non-Peak Season								
8. 23rd & Balboa	.11	.12	.11	.12	.11	.13	.11	.13
9. 21st & Balboa	.23	.32	.23	.32	.23	.33	.23	.33
Summer Season								
8. 23rd & Balboa	.13	.14	.13	.14	.13	.15	.13	.15
9. 21st & Balboa	.28	.36	.28	.36	.29	.37	.29	.37
I1 -f: 00 00 A								
Level of service ranges: .0060 A .6170 B								
.6170 B .7180 C								
.7180 C .8190 D								
.91 – 1.00 E								
Above 1.00 F								
Above 1.001								

Table 8										
CUMULATIVE PROJECTS SUMMARY										
PROJECT	PROJECT LAND USE AMOUNT									
Newport Beach Country Club	Residential Hotel Tennis/Golf Club	5 DU 27 Rms 51.3 TSF								
Mariner's Medical Arts	Medical Office Addition	12.2 TSF								
City Hall & Park Development	City Hall Library Expansion	98.0 TSF 17.1 TSF								
WPI-Newport, LLC	Office/Retail	54.2 TSF								
Banning Ranch	Residential Retail Hotel	1,375 DU 75.0 TSF 75 Rms								
Sunset Ridge Park	Park	13.7 Acre								
Old Newport GPA	Medical Office	25.7 TSF								
Marina Park	Public Marina/Park	10.5 Acre								
Pres Office Building B	Office	16.7 TSF								
Conexant/Koll	Residential	974 DU								
AERIE	Condominium	6 DU								
Newport Coast TAZ 1 – 4	Single Family Detached Condominium/Townhouse Multi-Family Attached	954 DU 389 DU 175 DU								
DU – dwelling units Rms – Rooms TSF – thousand square feet										

cumulative project was also provided by City Staff. The peak hour cumulative intersection volumes were added to the background volumes presented earlier, and then Phase 3 project-generated traffic was added (see Appendix A). The previous non-peak season one percent analysis without cumulative volumes represents the worst-case one percent analysis since the addition of cumulative traffic to the background volumes increases the chances of a project passing the one percent analysis. If an intersection passes the one percent analysis prior to the addition of cumulative traffic, then the intersection will pass the one percent analysis with the addition of cumulative traffic and no further analysis is required at that location. Therefore, an ICU analysis for the two study intersections that did not pass the non-peak season one percent analysis was prepared.

The results of the cumulative ICU analysis are summarized in Table 9 (actual ICU calculation sheets are included in Appendix B). This table includes the non-peak season and Summer season ICU values. As the cumulative ICU table indicates, Phase 3 of the proposed project will have no significant impact on the study intersections which operate at LOS "A" during the AM and PM peak hours, and no mitigation is required.

PARKING ANALYSIS

The existing mobile home park parking lot will be restriped for Phase 1 and Phase 2 of the proposed project to provide approximately 112 metered parking spaces. Access to Phase 1 and Phase 2 parking will be provided via a driveway on 18th Street, as well as a connection to 15th Street at the east end of the parking lot.

Parking for Phase 3 of the proposed project will be provided in a main parking lot adjacent to the Community Center, Sailing Center, and Visitor Marina. This parking lot will provide approximately 127 spaces with two-way circulation throughout the lot. Access to the main parking lot will be provided at 16th Street and with a connection to 15th Street. Parking for the Girl Scout House will be provided in a 26-space lot at 18th Street. One-way circulation should be provided in the 18th Street parking lot with traffic entering the south driveway and exiting the north driveway.

The amount of parking required for Phase 3 of the proposed project was determined from ITE parking rates modified to suit this specific development. The parking rate applied to the Community Center is the ITE Recreational Community Center parking rate; however, the rate applied to the Sailing Center is reduced from the ITE Recreational Community Center rate since large areas of the sailing center

			Table 9					
CUMULATIVE ICU ANALYSIS SUMMARY – PHASE 3								
					BACKGF	ROUND +		ROUND + ATIVE +
	EXIS	TING	BACKG	ROUND	CUMUI	LATIVE	PROJECT	PHASE 3
INTERSECTION	AM	PM	AM	PM	AM	PM	AM	PM
Non-Peak Season 8. 23rd & Balboa 9. 21st & Balboa	.11 .23	.12 .32	.11 .23	.13 .33	.12 .24	.14	.12 .24	.14 .33
Summer Season 8. 23rd & Balboa 9. 21st & Balboa	.13 .28	.14 .36	.13 .29	.15 .37	.14 .29	.16 .38	.14 .29	.16 .38
Level of service ranges: .0060 .6170 .7180 .8190 .91 - 1.00 Above 1.00	B C D E							

are used for storage and maintenance. The parking rate for the Visitor Marina is the ITE Marina parking rate. These rates were developed in coordination with City staff.

Table 10 summarizes the required project parking for Phase 3. As this table indicates, the worst-case parking estimate for Phase 3 of the project is 144 spaces. Approximately 127 spaces will be provided in the main project parking lot under Phase 3, with another 26 spaces provided in the 18th Street parking lot for a total of 153 spaces.

The City is in the process of acquiring the existing SCE substation on Balboa Boulevard. The proposed Phase 3 site plan shows parking on the substation property. If the City is unsuccessful in obtaining the SCE property and cannot include that land in the parking lot, the parking lot will lose approximately 11 spaces resulting in an overall parking deficiency of two spaces under Phase 3 of the project.

The proposed Phase 3 parking lot is intended for the project only and not to provide additional beach parking. To prevent the parking lot from being used for beach parking under Phase 3, as a result reducing the amount of parking available for the project uses, signs indicating appropriate users and time limits and warning signs indicating enforcement will be posted throughout the parking lot. Periodic monitoring and surveying of parking lot users will be performed to determine if beach parking is encroaching on the project parking lot. If the warning signs are discouraging inappropriate users, then no further actions will be required. If the warning signs are being ignored, then a parking management plan with a more rigorous enforcement program will be developed.

Currently, on-street parking is allowed along Balboa Boulevard in the vicinity of the project site. Parking along the project frontage (i.e., the north side of Balboa Boulevard) is not metered; however, parking is not allowed on Monday mornings (8:30 AM – 12:30 PM) for street sweeping. Furthermore, parking is prohibited from 9:00 AM to 6:00 PM on Saturdays and Sundays from May through September, Memorial Day, Fourth of July, and Labor Day to provide an additional travel lane. These parking restrictions are assumed to remain in place with the development of the proposed project. Metered parking spaces are provided in the center median along Balboa Boulevard, and development of the project will have no affect on the operation of these spaces. A public parking lot with 24 spaces is currently located at the corner of 18th Street and Balboa Boulevard. This parking lot will be removed and replaced with a 26-space lot in the same location with development of Phase 3 of the project.

Table 10

PARKING SUMMARY – PHASE 3

LAND USE	SIZE	PARKING RATE	RATE SOURCE	SPACES REQUIRED
Girl Scout House Community Ctr Sailing Ctr Visitor Marina	5.5 TSF 10.2 TSF 11.1 TSF 23 Berths	2.36 sp/TSF 6 sp/TSF 5 sp/TSF .59 sp/Berth	1 2 3 4	13 sp 61 sp 56 sp 14 sp
TOTAL SPACES				144 sp

- Parking rate sources:

 1 Existing 6 spaces plus 7 added for expansion of facility

 2 ITE Recreational Community Center (495) includes Café and Park uses

 3 Modified ITE Recreational Community Center (495)

 4 ITE Marina (420)

The number of on-street parking spaces along the project frontage will change with the development of Phase 3 of the proposed project. Development of the project site under Phase 3 will result in the widening of 18th Street north of Balboa Boulevard which will allow additional on-street parking (approximately eight spaces). In addition, development of Phase 3 of the project will result in the closure of driveways on Balboa Boulevard along the project frontage at the public parking lot on the corner of 18th Street, the existing community center, Girl Scout House, and SCE substation, and the removal of the 30 minute loading zone in front of the existing community center. With the closure of these driveways under Phase 3, an additional four on-street parking spaces may be available; although, a new on-street drop-off zone may reduce this number of new on-street parking spaces. On the other hand, the need to provide adequate sight distance east of the project driveway at 16th Street will require the removal of approximately 15 existing on-street parking spaces.

A left-turn pocket from eastbound Balboa Boulevard into the parking lot driveway at 16th Street should be provided to reduce entering Phase 3 project traffic from blocking through traffic on Balboa Boulevard. Although the Phase 3 hourly volume entering the parking lot driveway is not large (approximately 11 to 17 vehicles per hour), project trips are not spread out evenly throughout the hour, with the majority of vehicles entering the parking lot in the 10 to 15 minutes before classes begin. An 85-foot pocket, similar to existing left-turn pockets on Balboa Boulevard in the project vicinity, would result in the loss of approximately three existing parking spaces in the center median on Balboa Boulevard.

The overall change in on-street parking as a result of Phase 3 of the proposed project is a reduction of approximately six spaces between 18th Street and 15th Street.

SPECIAL ISSUES

Access to the main parking lot of Phase 3 of the proposed project will be provided by a driveway opposite 16th Street. A connection to 15th Street will also be available. A traffic signal exists at the intersection of 15th Street and Balboa Boulevard, approximately 400 feet east of the proposed driveway. Installation of a traffic signal is being considered at the proposed driveway opposite 16th Street. The need for a signal at this location was examined.

Satisfaction of Caltrans Peak Hour Signal Warrant (Figure F-1) requires a minimum of 100 peak hour trips on the side street approach. Phase 3 of the proposed project will generate 38 AM peak hour driveway trips, 15 of which are outbound, and 46 PM peak hour driveway trips, 31 of which are

outbound. Phase 3 of the project does not generate enough peak hour traffic to satisfy the signal warrant. Besides, the majority of outbound trips from Phase 3 of the project will be oriented toward the west; therefore, they will make a right turn out of the driveway and will experience very little benefit from a traffic signal at this location. Installation of a signal at the proposed project driveway is not recommended due to the low peak hour volumes expected and the close proximity to the existing signal at 15th Street.

Without a signal at the proposed project driveway, outbound vehicles will have to wait for gaps in Balboa Boulevard traffic to exit. Vehicles making a left turn from the parking lot will have the option of exiting the parking lot onto 15th Street and utilizing the existing signal at the intersection of 15th Street and Balboa Boulevard. However, relatively little Phase 3 project traffic (approximately 15 percent) is expected to be oriented toward the Balboa Peninsula east of the project site during the peak hours. Less than five peak hour trips would be added to the 15th Street/Balboa Boulevard intersection. The existing signal at 15th Street, which currently operates at LOS "A" during the AM and PM peak hours, can accommodate the addition of five peak hour trips.

Without a signal at the proposed project driveway on Balboa Boulevard, vehicles will have to wait for gaps in traffic on Balboa Boulevard. Balboa Boulevard is classified as a primary roadway in the vicinity of the project site. Parking and landscaping will need to be restricted east of the proposed project driveway so that a sight distance of 450 feet is provided per City Standard STD-110-L. This restriction will remove all on-street parking along the north side of Balboa Boulevard between 16th Street and 15th Street (approximately 15 spaces) under Phase 3.

CONCLUSIONS

Phase 1 and Phase 2 of the proposed project consists of replacing the existing mobile home park on the site with 3.83 acres of sand (Phase 1) or turf (Phase 2). No amenities other than temporary restroom facilities will be provided. The trips generated by the proposed sand or turf park are less than the trips generated by the existing mobile homes; therefore, Phase 1 and Phase 2 of the proposed project will have no significant impact on the study intersections. Approximately 112 metered parking spaces will be provided under Phase 1 and Phase 2.

Phase 3 of the proposed project, consisting of a 10,200 square foot community center, an 11,200 square foot sailing center, park uses, a 23-berth non-commercial visitor marina, and re-located on-site

Girl Scout House, will generate eight new AM peak hour trips, six new PM peak hour trips, and 261 new daily trips. The marginal impact of Phase 3 project traffic on the street system was determined at ten intersections in the vicinity. Two of the ten intersections did not pass the City's one percent analysis; however, Phase 3 of the project had no marginal impact on the ICU values at these two intersections, which will continue to operate at level of service (LOS) "A" during the AM and PM peak hours under non-peak season and Summer season conditions. Consequently, Phase 3 of the proposed project has no significant impact on the study intersections, and no additional intersection improvements are required.

The impact of traffic from known but not approved projects was included in a cumulative conditions analysis. Under cumulative non-peak season and Summer season conditions, Phase 3 of the project had no marginal impact during the AM or PM peak hour on the ICU values at the two intersections that did not pass the one percent analysis. Therefore, Phase 3 of the proposed project has no significant impact on the study intersections under cumulative conditions, and no intersection mitigation measures are required.

A review of the proposed parking reveals that the 153 spaces provided on-site are adequate to satisfy Phase 3 of the project's demand. However, some monitoring of the parking lot as outlined in a parking management plan will be required to ensure that the parking there is limited to legitimate Marina Park Phase 3 users.

APPENDIX A

PEAK HOUR INTERSECTION VOLUMES

PEAK HOUR PROJECT TRIPS - PHASE 3

Figure A-1

4751

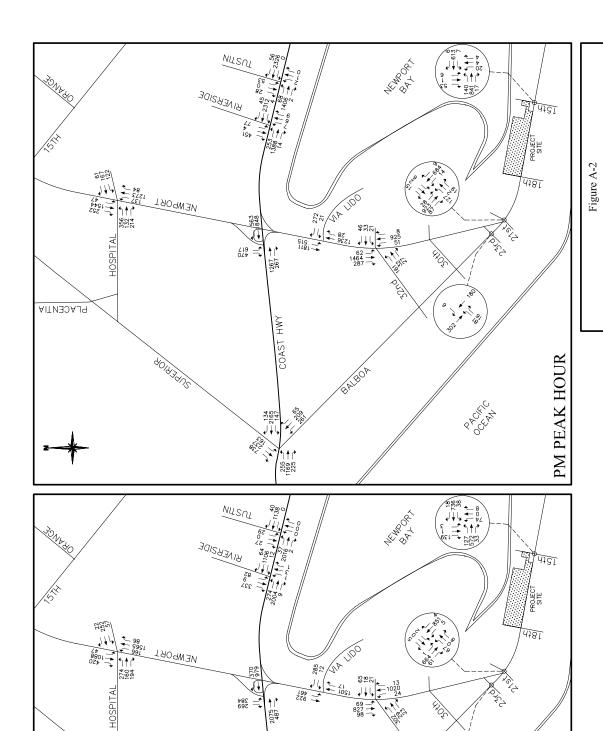
WITSUT

RINERSIDE

The bar

A-3

AM PEAK HOUR



COAST HWY

206 582 61

2242 238

BALBOA

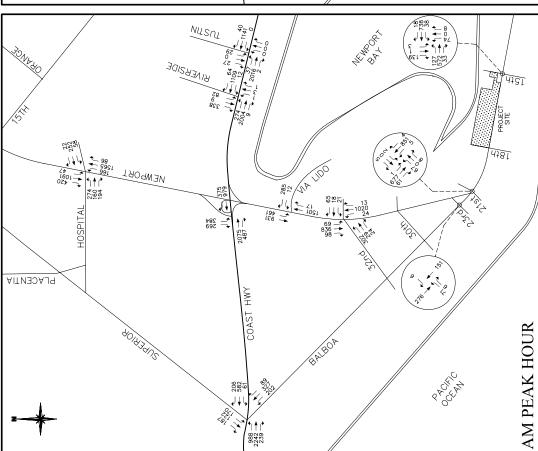
40143475

PLACENTIA

EXISTING PEAK HOUR VOLUMES - NON-PEAK SEASON

Marina Park TPO Traffic Analysis

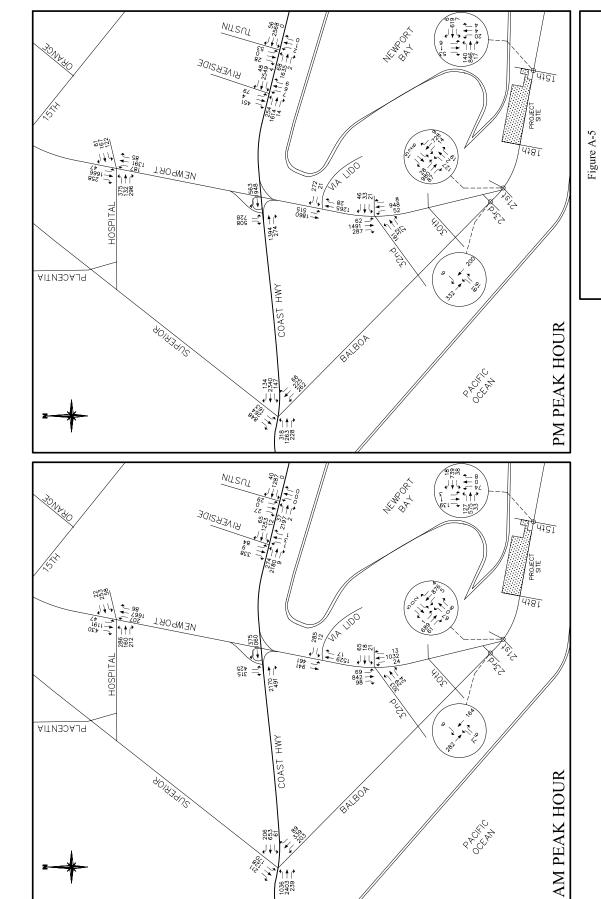


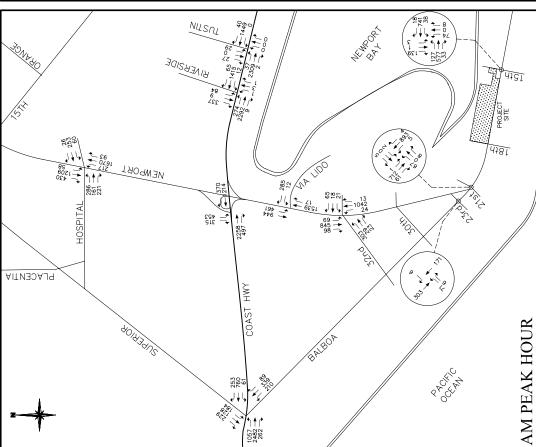


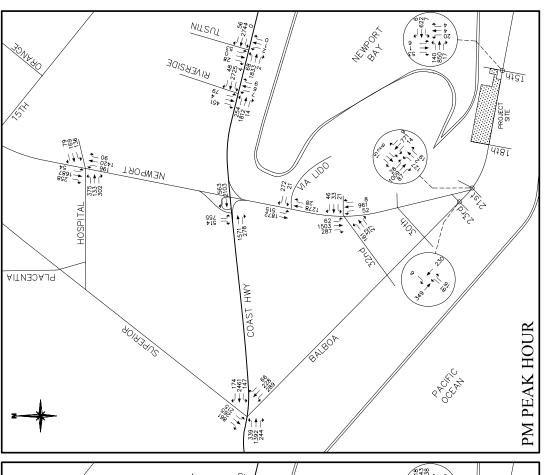
A-4

EXISTING + REGIONAL GROWTH + APPROVED
PEAK HOUR VOLUMES
- NON-PEAK SEASON

Figure A-4







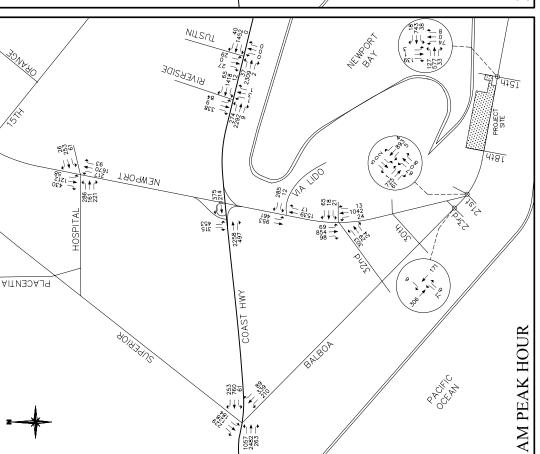


Figure A-7

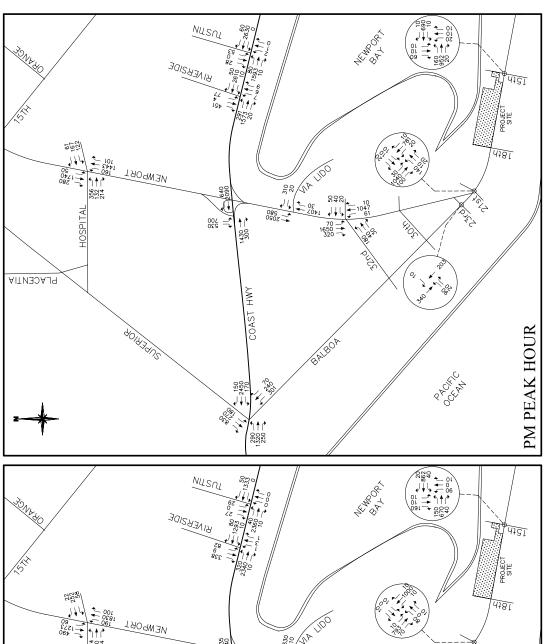
EXISTING + GROWTH + APPROVED + CUMULATIVE
+ PROJECT PHASE 3 PEAK HOUR VOLUMES
- NON-PEAK SEASON

EXISTING PEAK HOUR VOLUMES - SUMMER SEASON

Figure A-8

1160 2620 \$280

Marina Park TPO Traffic Analysis



AM PEAK HOUR

Figure A-10
EXISTING + REGIONAL GROWTH + APPROVED
PEAK HOUR VOLUMES
- SUMMER SEASON

Figure A-11
EXISTING + GROWTH + APPROVED
+ PROJECT PHASE 3 PEAK HOUR VOLUMES
- SUMMER SEASON

EXISTING + GROWTH + APPROVED + CUMULATIVE PEAK HOUR VOLUMES - SUMMER SEASON

Figure A-12

COAST HWY

287

1229 2860 305 BALBOA

40143475

PLACENTIA

0,

Figure A-13

EXISTING + GROWTH + APPROVED + CUMULATIVE
+ PROJECT PHASE 3 PEAK HOUR VOLUMES
- SUMMER SEASON

AM PEAK HOUR

APPENDIX B

INTERSECTION CAPACITY UTILIZATION

Peak hour intersection volume/capacity ratios are calculated by means of intersection capacity utilization (ICU) values. ICU calculations were performed for the intersections shown in Figure B-1. For simplicity, signalization is assumed at each intersection. Precise ICU calculations of existing non-signalized intersections would require a more detailed analysis.

The procedure is based on the critical movement methodology, and shows the amount of capacity utilized by each critical move. A capacity of 1600 vehicles per hour (VPH) per lane is assumed with no clearance interval. Calculations are carried out to three decimal places. A "de-facto" right-turn lane is used in the ICU calculation for cases where a curb lane is wide enough to separately serve both thru and right-turn traffic (typically with a width of 19 feet from curb to outside of thru-lane with parking prohibited during peak periods). Such lanes are treated the same as striped right-turn lanes during the ICU calculations, but they are denoted on the ICU calculation worksheets using the letter "d" in place of a numerical entry for right-turn lanes.

The methodology also incorporates a check for right-turn capacity utilization. Both right-turn-on-green (RTOG) and right-turn-on-red (RTOR) capacity availability are calculated and checked against the total right-turn capacity need. If insufficient capacity is available, then an adjustment is made to the total capacity utilization value. The following example shows how this adjustment is made.

Example For Northbound Right

1. Right-Turn-On-Green (RTOG)

```
If NBT is critical move, then: RTOG = V/C \; (NBT) Otherwise, RTOG = V/C \; (NBL) + V/C \; (SBT) - V/C \; (SBL)
```

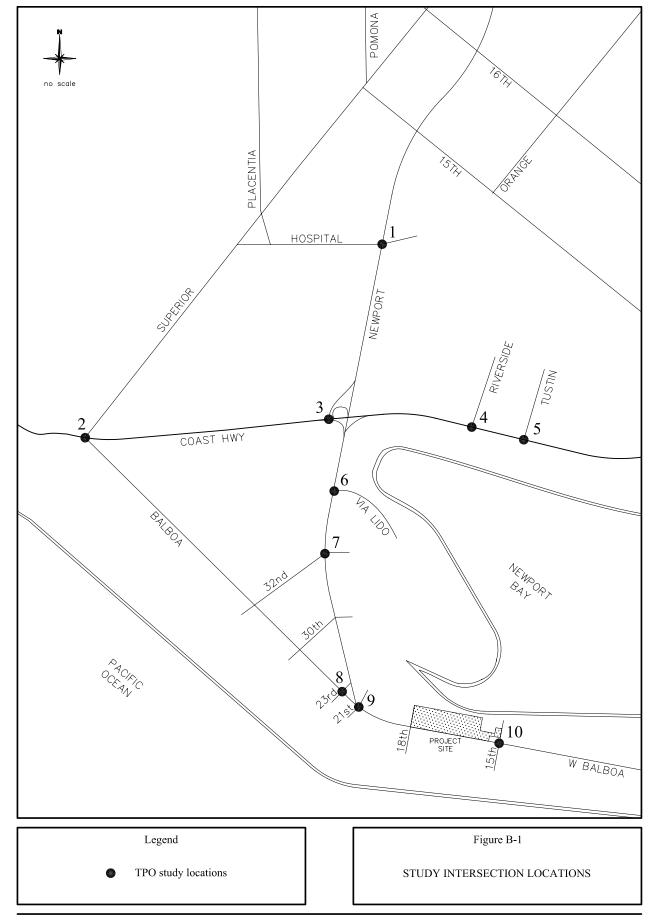
2. Right-Turn-On-Red (RTOR)

```
If WBL is critical move, then: 

RTOR = V/C \text{ (WBL)}

Otherwise, 

RTOR = V/C \text{ (EBL)} + V/C \text{ (WBT)} - V/C \text{ (EBT)}
```



3. Right-Turn Overlap Adjustment

If the northbound right is assumed to overlap with the adjacent westbound left, adjustments to the

RTOG and RTOR values are made as follows:

RTOG = RTOG + V/C (WBL)

RTOR = RTOR - V/C (WBL)

4. Total Right-Turn Capacity (RTC) Availability For NBR

 $RTC = RTOG + factor \times RTOR$

Where factor = RTOR saturation flow factor (75%)

Right-turn adjustment is then as follows: Additional ICU = V/C (NBR) - RTC

A zero or negative value indicates that adequate capacity is available and no adjustment is

necessary. A positive value indicates that the available RTOR and RTOG capacity does not adequately

accommodate the right-turn V/C, therefore the right-turn is essentially considered to be a critical

movement. In such cases, the right-turn adjustment is noted on the ICU worksheet and it is included in

the total capacity utilization value. When it is determined that a right-turn adjustment is required for more

than one right-turn movement, the word "multi" is printed on the worksheet instead of an actual right-turn

movement reference, and the right-turn adjustments are cumulatively added to the total capacity

utilization value. In such cases, further operational evaluation is typically carried out to determine if

under actual operational conditions, the critical right-turns would operate simultaneously, and therefore a

right-turn adjustment credit should be applied.

Shared Lane V/C Methodology

For intersection approaches where shared usage of a lane is permitted by more than one turn

movement (e.g., left/thru, thru/right, left/thru/right), the individual turn volumes are evaluated to

determine whether dedication of the shared lane is warranted to any one given turn movement. The

following example demonstrates how this evaluation is carried out:

Example for Shared Left/Thru Lane

1. Average Lane Volume (ALV)

ALV = <u>Left-Turn Volume + Thru Volume</u> Total Left + Thru Approach Lanes (including shared lane)

2. ALV for Each Approach

3. Lane Dedication is Warranted

If ALV (Left) is greater than ALV then full dedication of the shared lane to the left-turn approach is warranted. Left-turn and thru V/C ratios for this case are calculated as follows:

Similarly, if ALV (Thru) is greater than ALV then full dedication to the thru approach is warranted, and left-turn and thru V/C ratios are calculated as follows:

4. Lane Dedication is not Warranted

If ALV (Left) and ALV (Thru) are both less than ALV, the left/thru lane is assumed to be truly shared and each left, left/thru or thru approach lane carries an evenly distributed volume of traffic equal to ALV. A combined left/thru V/C ratio is calculated as follows:

$$V/C$$
 (Left/Thru) = Left-Turn Volume + Thru Volume
Total Left + Thru Approach Capacity (including shared lane)

This V/C (Left/Thru) ratio is assigned as the V/C (Thru) ratio for the critical movement analysis and ICU summary listing.

If split phasing has not been designated for this approach, the relative proportion of V/C (Thru) that is attributed to the left-turn volume is estimated as follows:

If approach has more than one left-turn (including shared lane), then: V/C (Left) = V/C (Thru)

If approach has only one left-turn lane (shared lane), then:

V/C (Left) = Left-Turn Volume

Single Approach Lane Capacity

If this left-turn movement is determined to be a critical movement, the V/C (Left) value is posted in brackets on the ICU summary printout.

These same steps are carried out for shared thru/right lanes. If full dedication of a shared thru/right lane to the right-turn movement is warranted, the right-turn V/C value calculated in step three is checked against the RTOR and RTOG capacity availability if the option to include right-turns in the V/C ratio calculations is selected. If the V/C value that is determined using the shared lane methodology described here is reduced due to RTOR and RTOG capacity availability, the V/C value for the thru/right lanes is posted in brackets.

When an approach contains more than one shared lane (e.g., left/thru and thru/right), steps one and two listed above are carried out for the three turn movements combined. Step four is carried out if dedication is not warranted for either of the shared lanes. If dedication of one of the shared lanes is warranted to one movement or another, step three is carried out for the two movements involved, and then steps one through four are repeated for the two movements involved in the other shared lane.

1. Newport & Hospital

Existing								
				C HOUR		K HOUR		
	LANES	CAPACITY	VOL	V/C	VOL	V/C		
NBL	1	1600	166	.104*	137	.086*		
NBT	3	4800	1565	.326	1273	.265		
NBR	1	1600	86	.054	84	.053		
SBL	1	1600	47	.029	47	.029		
SBT	3	4800	1088	.314*	1544	.374*		
SBR	0	0	420		252			
EBL	2	3200	274	.086*	356	.111*		
EBT	1	1600	160	.100	132	.083		
EBR	1	1600	194	.121	214	.134		
WBL	1	1600	57	.036	122	.076		
WBT	2	3200	252	.086*	167	.071*		
WBR	0	0	22		61			

Summer - Existing								
	LANES	CAPACITY	AM PK VOL	T HOUR V/C	PM PM VOL	V/C		
NBL NBT NBR	1 3 1	1600 4800 1600	190 1830 100	.119* .381 .063	160 1440 100	.100* .300 .063		
SBL SBT SBR	1 3 0	1600 4800 0	60 1270 490	.038	50 1740 280	.031 .421*		
EBL EBT EBR	2 1 1	3200 1600 1600	274 160 194	.086* .100 .121	356 132 214	.111* .083 .134		
WBL WBT WBR	1 2 0	1600 3200 0	57 252 22	.036	122 167 61	.076 .071*		

TOTAL CAPACITY UTILIZATION .590 .642

2. Balboa/Superior & Coast Hwy

Existing							
			AM PK	HOUR	PM Pk	K HOUR	
	LANES	CAPACITY	AOT	V/C	VOL	V/C	
NBL	1.5		202		261		
NBT	1.5	4800	327	.129*	209	.111*	
NBR	0		89		65		
SBL	2.5		170		163	.051	
SBT	1.5	6400	122	.046*	237	.074*	
SBR	2	3200	187	.058	738	.231	
EBL	2	3200	988	.309	255	.080*	
EBT	3	4800	2242	.467*	1169	.244	
EBR	1	1600	238	.149	225	.141	
WBL	1	1600	61	.038*	147	.092	
WBT	4	6400	582	.121	2165	.359*	
WBR	0	0	206	.129	134		
Right	Turn Ad	.justment			SBR	.097*	
		N/S Split	Phasing	ſ			

_		6400 3200 3200 4800 1600 1600 6400 0	200 140 220 1160 2620 280 70 680 240	.069 .363 .546* .175 .044* .142	290 1320 250	.259 .091* .275 .156
SBT SBR EBL EBT EBR WBL WBL WBT	1.5 2 2 3 1 1 4 0	3200 3200 4800 1600 1600 6400 0	140 220 1160 2620 280 70 680	.069 .363 .546* .175 .044* .142	270 830 290 1320 250 170 2450 150	.084* .259 .091* .275 .156 .106 .406*
SBT SBR EBL EBT EBR WBL WBL	1.5 2 2 3 1	3200 3200 4800 1600 1600 6400	140 220 1160 2620 280 70 680	.069 .363 .546* .175 .044* .142	270 830 290 1320 250 170 2450	.084* .259 .091* .275 .156
SBT SBR EBL EBT EBR	1.5 2 2 3 1	3200 3200 4800 1600	140 220 1160 2620 280	.069 .363 .546* .175	270 830 290 1320 250	.084* .259 .091* .275 .156
SBT SBR EBL EBT EBR	1.5 2 2 3 1	3200 3200 4800 1600	140 220 1160 2620 280	.069 .363 .546* .175	270 830 290 1320 250	.084* .259 .091* .275 .156
SBT SBR EBL EBT	1.5 2 2 3	3200 3200 4800	140 220 1160 2620	.069 .363 .546*	270 830 290 1320	.084* .259 .091* .275
SBT SBR EBL	1.5 2 2 3	3200 3200	140 220 1160	.363	270 830 290	.084* .259
SBT SBR	1.5	3200	140 220	.069	270 830	.084* .259
SBT	1.5		140		270	.084*
		6400		.053*		
SBL	2.5		200		180	.056
NBR	0		100		70	
NBT	1.5	4800	380	.150*	240	.127*
NBL	1.5		240		300	
	LANES	CAPACITY	VOL	V/C	VOL	V/C
			AM PK	HOUR	PM PK	HOUR
Summer	- Exis	ting				
	NBL NBT	LANES NBL 1.5 NBT 1.5	NBL 1.5 NBT 1.5 4800	AM PK LANES CAPACITY VOL NBL 1.5 240 NBT 1.5 4800 380	AM PK HOUR LANES CAPACITY VOL V/C NBL 1.5 240 NBT 1.5 4800 380 .150*	AM PK HOUR PM PK LANES CAPACITY VOL V/C VOL NBL 1.5 240 300 NBT 1.5 4800 380 .150* 240

3. Newport & Coast Hwy

Existing								
			AM PK	HOUR	PM Pk	K HOUR		
	LANES	CAPACITY	VOL	V/C	VOL	V/C		
NBL	0	0	0		0			
NBT	0	0	0		0			
NBR	0	0	0		0			
SBL	2	3200	384	.120*	617	.193*		
SBT	0	0	0		0			
SBR	1	1600	269	.168	470	.294		
EBL	0	0	0		0			
EBT	2	3200	2075	.648*	1267	.396*		
EBR	f		487		267			
 WBL	0	0	0		0			
WBT	3	4800	979	.204	1848	.385		
WBR	f		370		563			
 Right	Turn Ad	justment			SBR	.093*		

Summe	r - Exis	sting				
			AM PK	HOUR	PM Pk	K HOUR
	LANES	CAPACITY	VOL	V/C	VOL	V/C
NBL	0	0	0		0	
NBT	0	0	0		0	
NBR	0	0	0		0	
SBL	2	3200	450	.141*	700	.219*
SBT	0	0	0		0	
SBR	1	1600	320	.200	530	.331
EBL	0	0	0		0	
EBT	2	3200	2430	.759*	1430	.447*
EBR	f		570		300	
WBL	0	0	0		0	
WBT	3	4800	1140	.238	2090	.435
WBR	f		430		640	
Right	Turn Ad	ljustment			SBR	.103*

TOTAL CAPACITY UTILIZATION .768 .682

4. Riverside & Coast Hwy

Existing									
			AM P	AM PK HOUR		K HOUR			
	LANES	CAPACITY	VOL	V/C	VOL	V/C			
NBL	0	0	1	{.001}*	7				
NBT	1	1600	3		6	.014*			
NBR	0	0	1		9	İ			
SBL	0	0	82		77	{.048}*			
SBT	1	1600	9	.057*	4	, , ,			
SBR	1	1600	337	.211	451				
EBL	1	1600	274	.171	253	.158*			
EBT	2	3200	2004	.629*	1388	.438			
EBR	0	0	9		14				
 WBL	1	1600	12	.008*	4	.003			
WBT	3	4800	1106		2312				
WBR	1	1600	64	.040	45	.028			
Right	Turn Ad	justment			SBR	.106*			

Summe	r - Exis	ting				
		CA DA CIMU		K HOUR		K HOUR
	LANES	CAPACITY	AOT	V/C	VOL	V/C
NBL	0	0	1	{.001}*	7	
NBT	1	1600	3	.003	6	.014*
NBR	0	0	1		9	
SBL	0	0	82		77	{.048}*
SBT	1	1600	9	.057*	4	.051
SBR	1	1600	337	.211	451	.282
EBL	1	1600	320	.200	290	.181*
EBT	2	3200	2340	.734*	1570	.497
EBR	0	0	10		20	
 WBL	1	1600	10	.006*	10	.006
WBT	3	4800	1290	.269	2610	.544*
WBR	1	1600	80	.050	50	.031
Right	Turn Ad	justment			SBR	.088*

5. Tustin & Coast Hwy

Existi	ing					
			AM PK	HOUR	PM P	K HOUR
	LANES	CAPACITY	VOL	V/C	VOL	V/C
NBL	0	0	0		2	{.001}*
NBT	1	1600	0	.000	1	.002
NBR	0	0	0		0	
SBL	0	0	29		53	
SBT	1	1600	0	.035*	0	.051*
SBR	0	0	27		28	
EBL	1	1600	37	.023	68	.043*
EBT	2	3200	2016	.631*	1406	.440
EBR	0	0	2		2	
 WBL	0	0	0		0	
WBT	3	4800	1138	.237	2326	.485*
WBR	1	1600	40	.025	56	.035

Summer	- Exis	ting				
			AM PK	HOUR	PM Pl	K HOUR
	LANES	CAPACITY	VOL	V/C	VOL	V/C
NBL	0	0	0		2	{.001}*
NBT	1	1600	0	.000	1	.002
NBR	0	0	0		0	
SBL	0	0	29		53	
SBT	1	1600	0	.035*	0	.051*
SBR	0	0	27		28	
EBL	1	1600	40	.025	80	.050*
EBT	2	3200	2360	.741*	1590	.500
EBR	0	0	10		10	
WBL	0	0	0		0	
WBT	3	4800	1330	.277	2630	.548*
WBR	1	1600	50	.031	60	.038

TOTAL CAPACITY UTILIZATION .666 .580 TOTAL CAPACITY UTILIZATION .776

.650

6. Newport & Via Lido

Existing								
	LANES	CAPACITY	AM PR	HOUR V/C	PM PF	T HOUR V/C		
NBL NBT NBR	0 3 f	0 4800	0 1501 17	.313*	0 1236 28	.258*		
SBL SBT SBR	2 3 0	3200 4800 0	461 922 0	.144* .192	515 1811 0	.161* .377		
EBL EBT EBR	0 0 0	0 0 0	0 0 0		0 0 0			
WBL WBT WBR	1 0 2	1600 0 3200	12 0 285	.008*	21 0 272	.013*		

Summer - Existing								
	LANES	CAPACITY	AM Pk VOL	V/C	PM Pk VOL	V/C		
NBL NBT NBR	0 3 f	0 4800	0 1760 20	.367*	0 1400 30	.292*		
SBL SBT SBR	2 3 0	3200 4800 0	540 1080 0	.169* .225	580 2050 0	.181* .427		
EBL EBT EBR	0 0 0	0 0 0	0 0 0		0 0 0			
WBL WBT WBR	1 0 2	1600 0 3200	10 0 330	.006*	20 0 310	.013*		

TOTAL CAPACITY UTILIZATION .465 .432 TOTAL CAPACITY UTILIZATION .542 .486

7. Newport & 32nd

Existing									
	LANES	CAPACITY		HOUR V/C		HOUR V/C			
NBL NBT NBR	1 2 0	1600 3200 0	24 1020 13		51 925 8				
SBL SBT SBR	1 2 0	1600 3200 0	69 827 98	.043*		1			
EBL EBT EBR	1.5 0.5 f	3200	302 29 23	.103*	161 32 27	.060*			
WBL WBT WBR	0 2 f	0 3200	21 18 65	.012*	21 33 46	.017*			
Note:	Assumes	E/W Split	Phasing						

	Summer	- Exist	ting				
				AM PK	HOUR	PM PK	HOUR
		LANES	CAPACITY	VOL	V/C	VOL	V/C
1	NBL	1	1600	30	.019	60	.038*
1	NBT	2	3200	1190	.378*	1040	.328
1	NBR	0	0	20		10	
	SBL	1	1600	80	.050*	70	.044
5	SBT	2	3200	970	.341	1650	.616*
5	SBR	0	0	120		320	
	EBL	1.5		350		180	
I	EBT	0.5	3200	30	.119*	40	.069*
I	EBR	f		30		30	
1	WBL	0	0	20		20	
1 '	WBT	2	3200	20	.013*	40	.019*
	WBR	f	3200	80	.025	50	.025
1	Note:	Assumes	E/W Split	Phasing			

TOTAL CAPACITY UTILIZATION .481 .656 TOTAL CAPACITY

TOTAL CAPACITY UTILIZATION .560 .742

8. 23rd & Balboa

Existi	ing					
	LANES	CAPACITY	AM P	K HOUR V/C		HOUR V/C
NBL NBT NBR	1.5 0 0.5	3200		{.024}* .024	81 0 16	{.030}* .030
SBL SBT SBR	0 0 1	0 0 1600	0 0 6	.004	0 0 6	.004
EBL EBT EBR	0 2 0	0 3200 0	0 273 0	.085*	0 302 0	.094*
WBL WBT WBR	0 2 0	0 3200 0	0 151 0	.047	0 180 0	.056

.109 .124

TOTAL CAPACITY UTILIZATION

	LANES	CAPACITY	VOL	V/C	VOL	V/C	
NBL NBT NBR	1.5 0 0.5	3200	71 0 6	{.024}* .024	81 0 16	{.030}* .030	
SBL SBT SBR	0 0 1	0 0 1600	0 0 6	.004	0 0 6	.004	
EBL EBT EBR	0 2 0	0 3200 0	0 276 0	.086*	0 302 0	.094*	
WBL WBT WBR	0 2 0	0 3200 0	0 151 0	.047	0 183 0	.057	

.110

.124

AM PK HOUR PM PK HOUR

Existing + Project

TOTAL CAPACITY UTILIZATION

TOTAL CAPACITY UTILIZATION

Exist	ing + Re	gional Grow	th + A	pproved			
	TANDO	ON DA OTEN		K HOUR		HOUR	
	LANES	CAPACITY	VOL	V/C	VOL	V/C	
NBL	1.5		71	{.024}*	81	{.030}*	
NBT	0	3200	0	.024	0	.030	
NBR	0.5		6		16		
 SBL	0	0	0		0	I	
SBT	0	0	0		0	l	
SBR	1	1600	6	.004	6	.004	
SBK	1	1000	0	.004	Ö	.004	
EBL	0	0	0		0		
EBT	2	3200	279	.087*	332	.104*	
EBR	0	0	0		0		
WBL	0	0	٥		٥		
I MDD	U	U	U		U		

164 .051

.111

0

197

0

.062

.134

Exist	ing + Gr	owth + Appr	coved +	Project		
	LANES	CAPACITY	AM PI VOL	K HOUR V/C	PM PK	T HOUR V/C
NBL NBT NBR	1.5 0 0.5	3200		{.024}* .024	81 0 16	` '
SBL SBT SBR	0 0 1	0 0 1600	0 0 6	.004	0 0 6	.004
EBL EBT EBR	0 2 0	0 3200 0	0 282 0	.088*	0 332 0	.104*
WBL WBT WBR	0 2 0	0 3200 0	0 164 0	.051	0 200 0	.063

Marina Park TPO Traffic Analysis

WBT

WBR

2

0

TOTAL CAPACITY UTILIZATION

3200

0

.112

.134

8. 23rd & Balboa

Existing + Growth + Approved + Cumulative									
	LANES	CAPACITY		K HOUR V/C		HOUR V/C			
NBL NBT NBR	1.5 0 0.5	3200		{.024}* .024	81 0 16				
SBL SBT SBR	0 0 1	0 0 1600	0 0 6	.004	0 0 6	.004			
EBL EBT EBR	0 2 0	0 3200 0	0 303 0	.095*	0 349 0	.109*			
WBL WBT WBR	0 2 0	0 3200 0	0 171 0	.053	0 227 0	.071			

Existi	ing + Gr	owth + Appr	oved +	Cumulati	ve + Pr	oject
	LANES	CAPACITY	AM P	K HOUR V/C	PM PK VOL	HOUR V/C
NBL	1.5			{.024}*		{.030}*
NBT	0	3200	0	.024	0	.030
NBR	0.5		6		16	
SBL	0	0	0		0	
SBT	0	0	0		0	
SBR	1	1600	6	.004	6	.004
EBL	0	0	0		0	
EBT	2	3200	306	.096*	349	.109*
EBR	0	0	0		0	
WBL	0	0	0		0	
WBT	2	3200	171	.053	230	.072
WBR	0	0	0		0	

TOTAL CAPACITY UTILIZATION .119 .139

TOTAL CAPACITY UTILIZATION .120 .139

Summer	r - Exis	ting				
	LANES	CAPACITY		K HOUR V/C		HOUR V/C
NBL NBT NBR	1.5 0 0.5	3200		{.028}* .028	90 0 20	
SBL SBT SBR	0 0 1	0 0 1600	0 0 10	.006	0 0 10	.006
EBL EBT EBR	0 2 0	0 3200 0	0 320 0	.100*	0 340 0	.106*
WBL WBT WBR	0 2 0	0 3200 0	0 180 0	.056	0 200 0	.063

TOTAL CAPACITY UTILIZATION .128 .140

Summer	r - Exis	ting + Proj	ject			
	LANES	CAPACITY	AM P	K HOUR V/C	PM PK VOL	T HOUR V/C
NBL NBT NBR	1.5 0 0.5	3200	80 0 10	{.028}* .028	90 0 20	{.034}* .034
SBL SBT SBR	0 0 1	0 0 1600	0 0 10	.006	0 0 10	.006
EBL EBT EBR	0 2 0	0 3200 0	0 323 0	.101*	0 340 0	.106*
WBL WBT WBR	0 2 0	0 3200 0	0 180 0	.056	0 203 0	.063

TOTAL CAPACITY UTILIZATION .129 .140

8. 23rd & Balboa

Summer	Summer - Existing + Regional Growth + Approved								
	LANES	CAPACITY		K HOUR V/C		HOUR V/C			
NBL NBT NBR	1.5 0 0.5	3200		{.028}* .028	90 0 20				
SBL SBT	0 0 1	0	0 0	006	0 0	006			
SBR EBL EBT	0 2	1600 0 3200	10 0 326		10 0 370				
EBR WBL	0	0	0		0				
WBT WBR	2	3200	193 0	.060	217	.068			

Summe	r - Exis	ting + Grow	vth + A	pproved +	Projec	et
	LANES	CAPACITY	AM PI VOL	K HOUR V/C	PM PF VOL	T HOUR V/C
NBL NBT NBR	1.5 0 0.5	3200	80 0 10	{.028}* .028	90 0 20	{.034}* .034
SBL SBT SBR	0 0 1	0 0 1600	0 0 10	.006	0 0 10	.006
EBL EBT EBR	0 2 0	0 3200 0	0 329 0	.103*	0 370 0	.116*
WBL WBT WBR	0 2 0	0 3200 0	0 193 0	.060	0 220 0	.069
ТОТΆΙ.	CAPACTT	Y 11TT1.TZAT1		.131		.150

TOTAL	CAPACITY	UTILIZATION	.130	.150

OTAL CAPACITY UTILIZATION	.131	.150
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Summer	Summer - Existing + Growth + Approved + Cumulative						
	LANES	CAPACITY		K HOUR V/C		HOUR V/C	
NBL NBT NBR	1.5 0 0.5	3200		{.028}* .028		{.034}* .034	
SBL SBT SBR	0 0 1	0 0 1600	0 0 10	.006	0 0 10	.006	
EBL EBT EBR	0 2 0	0 3200 0	0 350 0	.109*	0 387 0	.121*	
WBL WBT WBR	0 2 0	0 3200 0	0 200 0	.063	0 247 0	.077	

	Summer - Existing + Growth + Approved + Cumulative + Project						
-	,		AM PI	K HOUR	PM PK	HOUR	
	LANES	CAPACITY	VOL	V/C	VOL	V/C	
NBL	1.5		80	{.028}*	90	{.034}*	
NBT	0	3200	0	.028	0	.034	
NBR	0.5		10		20		
SBL	0	0	0		0		
SBT	0	0	0		0		
SBR	1	1600	10	.006	10	.006	
EBL	0	0	0		0		
EBT	2	3200	353	.110*	387	.121*	
EBR	0	0	0		0		
WBL	0	0	0		0		
WBT	2	3200	200	.063	250	.078	
WBR	0	0	0		0		

TOTAL CAPACITY UTILIZATION .137 .155

TOTAL CAPACITY UTILIZATION .138 .155

9. 21st & Balboa

Existing							
	LANES	CAPACITY		K HOUR V/C	PM PK VOL		
	•		68		100		
NBL	0	0	67		127		
NBT	1	1600	0	.047*	2	.093*	
NBR	0	0	8		19		
SBL	0	0	2	{.001}*	6	{.004}*	
SBT	1	1600	0	.004	2		
SBR	0	0	5		15		
EBL	1	1600	1	.001*	28	.018	
EBT	3	4800	664	.151	923	.210*	
EBR	0	0	61		87		
WBL	1	1600	5	.003	14	.009*	
WBT	3	4800	851	.178*	684	.144	
WBR	0	0	4		9		

TOTAL CAPACITY UTILIZATION	.227	.316
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Existi	Existing + Regional Growth + Approved						
	TANTEO	ON DA OTTOW	AM P	K HOUR V/C	PM PK VOL	HOUR V/C	
	LANES	CAPACITY	VOL	V/C	νоп	V/C	
NBL	0	0	67		127		
NBT	1	1600	0	.047*	2	.093*	
NBR	0	0	8		19		
SBL	0	0	2	{.001}*	6	{.004}*	
SBT	1	1600	0	.004	2		
SBR	0	0	5		15	,,,,,	
EBL	1	1600	1	.001*	28	.018	
EBT	3	4800	676	.154	980	.222*	
EBR	0	0	61		87		
WBL	1	1600	5	.003	14	.009*	
WBT	3	4800	876	.183*	717	.151	
WBR	0	0	4		9		

TOTAL CAPACITY UTILIZATION	.232	.328
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Existing + Project							
	LANES	CAPACITY	AM PH VOL	K HOUR V/C	PM PK	HOUR V/C	
NBL NBT NBR	0 1 0	0 1600 0	67 0 8	.047*	127 2 19	.093*	
SBL SBT SBR	0 1 0	0 1600 0		{.001}* .004	6 2 15	{.004}* .014	
EBL EBT EBR	1 3 0	1600 4800 0	1 677 61		28 923 87	- 1	
WBL WBT WBR	1 3 0	1600 4800 0	5 851 4		14 695 9	1	

TOTAL CAPACITY	UTILIZATION	.227	.316

Exist	Existing + Growth + Approved + Project						
	LANES	CAPACITY	AM PH VOL	K HOUR V/C		HOUR V/C	
NBL NBT NBR	0 1 0	0 1600 0	67 0 8	.047*	127 2 19	.093*	
SBL SBT SBR	0 1 0	0 1600 0	2 0 5	{.001}* .004	6 2 15		
EBL EBT EBR	1 3 0	1600 4800 0	1 689 61		28 980 87	- 1	
WBL WBT WBR	1 3 0	1600 4800 0	5 876 4		14 728 9		

TOTAL CAPACITY UTILIZATION .232 .328

9. 21st & Balboa

Exist	Existing + Growth + Approved + Cumulative							
	LANES	CAPACITY	AM PI VOL	K HOUR V/C	PM PK VOL	HOUR V/C		
NBL	0	0	67		127			
NBT	1	1600	0	.047*	2	.093*		
NBR	0	0	8		19			
SBL	0	0	2	{.001}*	6	{.004}*		
SBT	1	1600	0	.004	2	.014		
SBR	0	0	5		15			
EBL	1	1600	1	.001*	28	.018		
EBT	3	4800	712	.161	1009	.228*		
EBR	0	0	61		87			
WBL	1	1600	5	.003	14	.009*		
WBT	3	4800	893	.187*	760	.160		
WBR	0	0	4		9			

NBL	0	0	67		127	
NBT	1	1600	0	.047*	2	.093*
NBR	0	0	8		19	
SBL	0	0	2	{.001}*	6	{.004}*
SBT	1	1600	0	.004	2	.014
SBR	0	0	5		15	İ
EBL	1	1600	1	.001*	28	.018
EBT	3	4800	725	.164	1009	.228*
EBR	0	0	61		87	
						į
WBL	1	1600	5	.003	14	.009*
WBT	3	4800	893	.187*	771	.163
WBR	0	0	4		9	
l						İ

Existing + Growth + Approved + Cumulative + Project

VOL

LANES CAPACITY

TOTAL CAPACITY UTILIZATION

Summer - Existing + Project

AM PK HOUR

V/C

.236

.006*

.180

.006

.210*

PM PK HOUR

V/C

.334

.019

.238*

.013*

.165

30

1040

100

20

781

10

VOL

TOTAL CAPACITY UTILIZATION .250	TOTAL	CAPACITY	UTILIZATION	.236
---------------------------------	-------	----------	-------------	------

0

0

0

1600

1600

4800

1600

4800

0

0

1600

AM PK HOUR

V/C

{.050}*

.013*

.006*

.177

.006

.210*

.056

VOL

80

0

10

10

0

10

10

780

70

10

10

1000

Summer - Existing

0

1

0

0

1

0

1

3

0

1

3

0

NBL

NBT

NBR

SBL

SBT

SBR

EBL

EBT

EBR

WBL

WBT

WBR

LANES CAPACITY

}*	

EBL

EBT

EBR

WBL

WBT

WBR

1

3

0

1

3

0

1600

4800

1600

4800

0

0

.334

PM PK HOUR

V/C

{.087

.106

.025*

.019

.238*

.013*

.163

 \mathtt{VOL}

140

10

20

10

10

20

30

1040

100

20

770

10

				K HOUR	PM PK	
	LANES	CAPACITY	VOL	V/C	VOL	V/C
NBL	0	0	80	{.050}*	140	{.087}
NBT	1	1600	0	.056	10	.106
NBR	0	0	10		20	
SBL	0	0	10		10	
SBT	1	1600	0	.013*	10	.025*
SBR	0	0	10		20	

10

793

70

10

10

1000

TOTAL CAPACITY UTILIZATION .279

.363

TOTAL CAPACITY UTILIZATION .279 .363

9. 21st & Balboa

Summer - Existing + Regional Growth + Approved								
	LANES	CAPACITY		K HOUR V/C	PM PK VOL	HOUR V/C		
NBL	0	0	80	{.050}*	140	{.087}*		
NBT	1	1600	0	.056	10	.106		
NBR	0	0	10		20			
SBL	0	0	10		10			
SBT	1	1600	0	.013*	10	.025*		
SBR	0	0	10		20			
EBL	1	1600	10	.006*	30	.019		
EBT	3	4800	792	.180	1097	.249*		
EBR	0	0	70		100			
WBL	1	1600	10	.006	20	.013*		
WBT	3	4800	1025	.216*	803	.169		
WBR	0	0	10		10			

TOTAL CAPACITY UTILIZATION	.285	.374
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Summer - Existing + Growth + Approved + Cumulative							
	LANES	CAPACITY	AM P	K HOUR V/C	PM PK	HOUR V/C	
NBL NBT NBR	0 1 0	0 1600 0	80 0 10	{.050}* .056		{.087}* .106	
SBL SBT SBR	0 1 0	0 1600 0	10 0 10	.013*	10 10 20	.025*	
EBL EBT EBR	1 3 0	1600 4800 0	10 828 70		30 1126 100		
WBL WBT WBR	1 3 0	1600 4800 0	10 1042 10		20 846 10	!	

TOTAL CAPACITY	UTILIZATION	.288	.380

Summer - Existing + Growth + Approved + Project							
	LANES	CAPACITY		K HOUR V/C		HOUR V/C	
NBL NBT NBR	0 1 0	0 1600 0	80 0 10	{.050}* .056		` '!	
SBL SBT SBR	0 1 0	0 1600 0	10 0 10	.013*	10 10 20	.025*	
EBL EBT EBR	1 3 0	1600 4800 0	10 805 70		30 1097 100	.019 .249*	
WBL WBT WBR	1 3 0	1600 4800 0	10 1025 10		20 814 10	.013*	

TOTAL CAPACITY	UTILIZATION	.285	.374

Summer - Existing + Growth + Approved + Cumulative + Project								
,			AM Pl	K HOUR	PM PK	HOUR		
	LANES	CAPACITY	VOL	V/C	VOL	V/C		
NBL	0	0	80	{.050}*	140	{.087}*		
NBT	1	1600	0	.056	10	.106		
NBR	0	0	10		20			
SBL	0	0	10		10			
SBT	1	1600	0	.013*	10	.025*		
SBR	0	0	10		20			
EBL	1	1600	10	.006*	30	.019		
EBT	3	4800	841	.190	1126	.255*		
EBR	0	0	70		100			
WBL	1	1600	10	.006	20	.013*		
WBT	3	4800	1042	.219*	857	.181		
WBR	0	0	10		10			

TOTAL CAPACITY UTILIZATION .288 .380

10. 15th & Balboa

Existing								
	LANES	CAPACITY	AM P	K HOUR V/C	PM PK	T HOUR V/C		
NBL NBT NBR	0 1 0	0 1600 0	74 0 8	{.046}* .051	20 4 4	` ,		
SBL SBT SBR	1 1 0	1600 1600 0	3 1 139	.002 .088*	6 1 53	.004		
EBL EBT EBR	1 2 0	1600 3200 0	127 572 33		140 841 17			
WBL WBT WBR	0 2 0	0 3200 0	38 736 18	.248*	7 613 6	.196*		

Summer	Summer - Existing											
	LANES	CAPACITY		K HOUR V/C	PM PK	HOUR V/C						
NBL NBT NBR	0 1 0	0 1600 0	90 0 10	{.056}* .063	20 10 10	{.012}* .025						
SBL SBT SBR	1 1 0	1600 1600 0	10 10 160		10 10 60							
EBL EBT EBR	1 2 0	1600 3200 0	150 670 40		160 950 20							
WBL WBT WBR	0 2 0	0 3200 0	40 860 20	.288*	10 690 10	.222*						

TOTAL CAPACITY UTILIZATION .461 .330 TOTAL CAPACITY UTILIZATION

OTAL CAPACITY UTILIZATION .544 .378

APPENDIX C

1% ANALYSIS WORKSHEETS

Intersection:

1. Newport Blvd & Hospital Rd

Existing Traffic Volumes Based on Average Winter/Spring 2008

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume
			AM PE	AK PERIOD			
Northbound	1817	55	96	0	1968	20	0
Southbound	1555	47	78	0	1680	17	3
Eastbound	628	0	30	0	658	7	0
Westbound	331	0	1	0	332	3	1

^{==&}gt; Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	1494	45	127	0	1666	17	4
Southbound	1843	55	82	0	1980	20	0
Eastbound	702	0	102	0	804	8	0
Westbound	350	0	0	0	350	4	0

^{==&}gt; Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

Intersection: 2. Balboa Blvd/Superior Ave & Coast Hwy
Existing Traffic Volumes Based on Average Winter/Spring 200

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume
			AM PE	AK PERIOD			
Northbound	618	0	13	0	631	6	0
Southbound	479	0	30	23	532	5	0
Eastbound	3468	173	97	46	3784	38	1
Westbound	849	42	42	231	1164	12	0

^{==&}gt; Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	535	0	17	0	552	6	1
Southbound	1138	0	137	79	1354	14	0
Eastbound	1649	82	102	158	1991	20	0
Westbound	2446	122	67	142	2777	28	0

^{==&}gt; Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

Intersection:

3. Newport Blvd & Coast Hwy

Existing Traffic Volumes Based on Average Winter/Spring 2007

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume
			AM PE	AK PERIOD			
Northbound	0	0	0	0	0	0	0
Southbound	653	26	61	0	740	7	0
Eastbound	2562	102	16	67	2747	27	0
Westbound	1098	44	42	231	1415	14	5

^{==&}gt; Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	0	0	0	0	0	0	0
Southbound	1087	43	105	0	1235	12	0
Eastbound	1534	61	83	238	1916	19	0
Westbound	2411	96	26	142	2675	27	0

^{==&}gt; Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

Intersection:

4. Riverside Ave & Coast Hwy

Existing Traffic Volumes Based on Average Winter/Spring 2008

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume
			AM PE	AK PERIOD			
Northbound	5	0	0	0	5	0	0
Southbound	428	0	2	0	430	4	1
Eastbound	2287	69	116	67	2539	25	0
Westbound	1182	35	112	231	1560	16	3

^{==&}gt; Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	22	0	0	0	22	0	0
Southbound	532	0	2	0	534	5	0
Eastbound	1655	50	181	238	2124	21	4
Westbound	2361	71	171	142	2745	27	0

^{==&}gt; Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

Intersection:

5. Tustin Ave & Coast Hwy

Existing Traffic Volumes Based on Average Winter/Spring 2008

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume
			AM PE	AK PERIOD			
Northbound	0	0	0	0	0	0	0
Southbound	56	0	0	0	56	1	0
Eastbound	2055	62	121	67	2305	23	0
Westbound	1178	35	112	231	1556	16	3

^{==&}gt; Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	3	0	0	0	3	0	0
Southbound	81	0	0	0	81	1	0
Eastbound	1476	44	184	238	1942	19	3
Westbound	2382	71	172	142	2767	28	0

^{==&}gt; Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

Intersection:

6. Newport Blvd & Via Lido

Existing Traffic Volumes Based on Average Winter/Spring 2007

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume
			AM PE	AK PERIOD			
Northbound	1518	0	28	0	1546	15	0
Southbound	1383	0	10	0	1393	14	9
Eastbound	0	0	0	0	0	0	0
Westbound	297	0	0	0	297	3	0

^{==&}gt; Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	1264	0	22	0	1286	13	7
Southbound	2326	0	49	0	2375	24	0
Eastbound	0	0	0	0	0	0	0
Westbound	293	0	0	0	293	3	0

^{==&}gt; Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

Intersection:

7. Newport Blvd & 32nd St

Existing Traffic Volumes Based on Average Winter/Spring 2007

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume			
AM PEAK PERIOD										
Northbound	1057	0	12	0	1069	11	0			
Southbound	994	0	6	0	1000	10	9			
Eastbound	354	0	1	0	355	4	1			
Westbound	104	0	0	0	104	1	0			

^{==&}gt; Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	984	0	16	0	1000	10	8
Southbound	1813	0	27	0	1840	18	0
Eastbound	220	0	0	0	220	2	0
Westbound	100	0	0	0	100	1	0

^{==&}gt; Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

Intersection:

8. 23rd St & Balboa Blvd

Existing Traffic Volumes Based on Average Winter/Spring 2009

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume			
AM PEAK PERIOD										
Northbound	77	0	0	0	77	1	0			
Southbound	6	0	0	0	6	0	0			
Eastbound	273	0	6	0	279	3	3			
Westbound	151	0	13	0	164	2	0			

Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	97	0	0	0	97	1	0
Southbound	6	0	0	0	6	0	0
Eastbound	302	0	30	0	332	3	0
Westbound	180	0	17	0	197	2	3

Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

1% Traffic Volume Analysis

Intersection:

9. 21st St & Balboa Blvd

Existing Traffic Volumes Based on Average Winter/Spring 2009

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume
			AM PE	AK PERIOD			
Northbound	75	0	0	0	75	1	0
Southbound	7	0	0	0	7	0	0
Eastbound	726	0	12	0	738	7	13
Westbound	860	0	25	0	885	9	0

Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume.

Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	148	0	0	0	148	1	0
Southbound	23	0	0	0	23	0	0
Eastbound	1038	0	57	0	1095	11	0
Westbound	707	0	33	0	740	7	11

Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PROJECT: Marina Park FULL OCCUPANCY YEAR: 2011

1% Traffic Volume Analysis

Intersection:

10. 15th St & Balboa Blvd

Existing Traffic Volumes Based on Average Winter/Spring 2009

Approach Direction	Existing Peak 1 Hour Volume	Peak 1 Hour Regional Growth Volume	Approved Projects Peak 1 Hour Volume	Cumulative Projects Peak 1 Hour Volume	Projected Peak 1 Hour Volume	1% of Projected Peak 1 Hour Volume	Project Peak 1 Hour Volume
			AM PE	AK PERIOD			
Northbound	82	0	0	0	82	1	0
Southbound	143	0	0	0	143	1	0
Eastbound	732	0	3	0	735	7	0
Westbound	792	0	1	0	793	8	2

^{==&}gt; Project AM Traffic is estimated to be less than 1% of Projected AM Peak 1 Hour Traffic Volume.

Project AM Traffic is estimated to be 1% or greater of Projected AM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PM PEAK PERIOD

Northbound	28	0	0	0	28	0	0
Southbound	60	0	0	0	60	1	0
Eastbound	998	0	3	0	1001	10	2
Westbound	626	0	6	0	632	6	0

^{==&}gt; Project PM Traffic is estimated to be less than 1% of Projected PM Peak 1 Hour Traffic Volume.

Project PM Traffic is estimated to be 1% or greater of Projected PM Peak 1 Hour Traffic Volume. Intersection Capacity Utilization (ICU) Analysis is required.

PROJECT: Marina Park FULL OCCUPANCY YEAR: 2011

APPENDIX D COUNT DATA

LOCATION - NEWPORT-BTN 32ND/31ST

VOLUMES FOR - TUESDAY 6/24/08

LOCATIO	1 11	MI OKT -BT	N OZNO	0131							VOI	LOPILS	OIX - IC	JESDAT 07	24700
			•			*******								******	
TIN		N ******			SB ******		TAL *****	 ********	ME *****		IB ******		B ******		TAL
12:00 -		35		48		83	.,	12:00 -		220		329		549	
12:15 -		47		35		82		12:15 -		291		350		641	
12:30 -		60		30		90		12:30 -		264		378		642	
12:45 -		44	186	32	145	76	331	12:45 -			1029	354	1411		2440
1:00 -		40		26		66		1:00 -		296		322		618	
1:15 -		34		20		54		1:15 -		282		290		572	
1:30 -		37		22		59		1:30 -		260		294		554	
1:45 -	2:00	23	134	16	84	39	218	1:45 -	2:00	288	1126	294	1200	582	2326
2:00 -	2:15	13		14		27		2:00 -	2:15	271		286		557	
2:15 -	2:30	11		14		25		2:15 -	2:30	253		280		533	
2:30 -	2:45	4		6		10		2:30 -	2:45	268		316		584	
2:45 -	3:00	6	34	9	43	15	77	2:45 -	3:00	304	1096	282	1164	586	2260
3:00 -	3:15	8		8		16		3:00 -	3:15	304		242		546	
3:15 -	3:30	6		4		10		3:15 -		290		261		551	
3:30 -		12		4		16		3:30 -		307		283		590	
3:45 -		9	35	10	26	19	61	3:45 -			1185	296	1082	580	2267
4.00	4 15	10		_		10		4.00	4 15	200		010		COO	
4:00 -		13		5		18		4:00 -		326		312 312		638	
4:15 -		11 7		8		19		4:15 -		320 288		274		632 562	
4:30 - 4:45 -		17	48	4 28	45	11 45	93	4:30 - 4:45 -		262	1196	296	1194	558	2390
4.40 -	5.00	17	40	20	45	45	93	4.40 -	3.00	202	1190	290	1134	556	2390
5:00 -	5:15	23		31		54		5:00 -	5:15	288		343		631	
5:15 -	5:30	22		45		67		5:15 -	5:30	238		304		542	
5:30 -	5:45	45		64		109		5:30 -	5:45	286		364		650	
5:45 -	6:00	47	137	66	206	113	343	5:45 -	6:00	290	1102	349	1360	639	2462
6:00 -	6.15	58		68		126		6:00 -	6:15	294		317		611	
6:15 -		86		92		178		6:15 -		246		366		612	
6:30 -		98		100		198		6:30 -		268		306		574	
6:45 -		118	360	124	384	242	744	6:45 -		266	1074	292	1281	558	2355
7.00	7.1E	116		120		204		7.00	7.15	285		324		609	
7:00 - 7:15 -		146 184		138 138		284 322		7:00 - 7:15 -		256		288		544	
	7:45	220		160		380		7:13 -		246		252		498	
7:45 -		188	738	205	641		1379	7:45 -		243	1030	272	1136	515	2166
7.10	0.00	100	700	200	011	030	1075	7.10	0.00	210	1000	2,2	1100	010	2100
8:00 -	8:15	213		195		408		8:00 -	8:15	238		206		444	
8:15 -		247		214		461		8:15 -		229		214		443	
8:30 -	8:45	247		256		503		8:30 -		209		186		395	
8:45 -	9:00	262	969	250	915	512	1884	8:45 -	9:00	216	892	186	792	402	1684
9:00 -	9:15	244		232		476		9:00 -	9:15	203		160		363	
9:15 -	9:30	201		196		397		9:15 -	9:30	208		168		376	
9:30 -	9:45	199		208		407		9:30 -	9:45	210		154		364	
9:45 -	10:00	229	873	250	886	479	1759	9:45 -	10:00	186	807	147	629	333	1436
10:00 -	10.15	200		254		454		10:00 -	10.15	166		151		317	
10:15 -		184		270		454		10:00 -		164		147		311	
10:30 -		204		258		462		10:30 -		145		98		243	
10:45 -		212	800	270	1052		1852	10:45 -		133	608	95	491	228	1099
11.00	77.75	000		000		F00		11.00	11.15	116		70		104	
11:00 -		200		308		508		11:00 -		116		78 66		194	
11:15 -		232		295		527 501		11:15 -		84 73		66 62		150 135	
11:30 - 11:45 -		246 225	903	345 362	1310	591 587	2213	11:30 - 11:45 -		73 78	351	62 74	280	155 152	631
******		*****	*****	*****	*****	*****	*****	******		******	*****		*****	*****	****
TOTALS		į	5,217		5,737	10),954			1	1,496	1	2,020	2	3,516
ADT'S										1	6,713	1	7,757	3	4,470
*****	*****	******	*****	*****	*****	*****	*****	*****	*****						

LOCATION - BALBOA-BTN 19TH/18TH

VOLUMES FOR - TUESDAY 6/24/08

THE EB WB DTOPAL THE EB WB DTOPAL THE EB WB DTOPAL THE T	*****	******	*****	***** /	ΔM ****	*****	******	*****	****	*****	*****	****	DM ****	*****	*****	****
12:15 12:30 30	*****	*****	*****	*****		*****		*****	*****	****	*****	*****	*****	*****	*****	****
12,30 - 12,46																
12.45 - 1.00																
1:00 - 1:15								005						0.40		0.7.50
1:15 - 1:30	12:45 -	1:00	22	112	32	123	54	235	12:45	- 1:00	285	1211	245	949	530	2160
1:15 - 1:30	1.00	1.15	22		oc		40		1.00	1.15	046		040		400	
1:30 - 1:45																
1.45 - 2.00																
2:100 - 2:15				70		70		140				05.0		011		1860
2:15 - 2:30	1.40	2.00	17	70	14	70	OI	140	1.40	2.00	272	500	LLT	711	700	1005
2:15 - 2:30	2:00 -	2:15	12		9		21		2:00	- 2:15	225		234		459	
2:30 - 2:45																
2.45 - 3.00																
3:15 - 3:30	2:45 -	3:00	8	36	5	24	13	60	2:45	- 3:00	205	916		917	457	1833
3:15 - 3:30																
3:30 - 3:45	3:00 -	3:15			6		11		3:00	- 3:15			240			
3:45 - 4:00	3:15 -	3:30			4		9		3:15	- 3:30	252		241		493	
4:100 - 4:155					4											
4:15 - 4:30	3:45 -	4:00	5	17	4	18	9	35	3:45	- 4:00	254	951	236	969	490	1920
4:15 - 4:30			_													
4:45 - 5:00																
4:45 - 5:00																
5:00 - 5:15								0.1				071		10.47		0010
5:15 - 5:30 30 23 53 5:15 - 5:30 262 206 468 5:30 - 5:45 - 6:00 46 132 48 133 94 265 5:45 - 6:00 266 218 484 484 551 2033 6:00 - 6:15 266 218 484 484 551 2033 6:00 - 6:15 266 273 533 2033 6:05 - 6:15 - 6:30 274 187 461 6:15 - 6:30 274 187 461 6:30 - 6:45 - 7:00 83 239 102 316 185 555 6:45 - 7:00 232 976 187 461 6:45 - 7:00 83 239 102 316 185 555 6:45 - 7:00 232 976 187 867 419 1843 7:00 - 7:15 104 130 234 7:00 - 7:15 237 202 439 7:15 - 7:30 102 117 7:45 - 7:10 232 190 413 7:30 - 7:15 - 7:30 222 139 411 <t< td=""><td>4:45 -</td><td>5:00</td><td>23</td><td>38</td><td>14</td><td>43</td><td>3/</td><td>81</td><td>4:45</td><td>- 5:00</td><td>224</td><td>9/1</td><td>228</td><td>1047</td><td>452</td><td>2018</td></t<>	4:45 -	5:00	23	38	14	43	3/	81	4:45	- 5:00	224	9/1	228	1047	452	2018
5:15 - 5:30 30 23 53 5:15 - 5:30 262 206 468 5:30 - 5:45 - 6:00 46 132 48 133 94 265 5:45 - 6:00 266 218 484 484 551 2033 6:00 - 6:15 266 218 484 484 551 2033 6:00 - 6:15 266 273 533 2033 6:05 - 6:15 - 6:30 274 187 461 6:15 - 6:30 274 187 461 6:30 - 6:45 - 7:00 83 239 102 316 185 555 6:45 - 7:00 232 976 187 461 6:45 - 7:00 83 239 102 316 185 555 6:45 - 7:00 232 976 187 867 419 1843 7:00 - 7:15 104 130 234 7:00 - 7:15 237 202 439 7:15 - 7:30 102 117 7:45 - 7:10 232 190 413 7:30 - 7:15 - 7:30 222 139 411 <t< td=""><td>E.00</td><td>C.1C</td><td>20</td><td></td><td>22</td><td></td><td>40</td><td></td><td>E . 00</td><td>E.1E</td><td>200</td><td></td><td>242</td><td></td><td>E30</td><td></td></t<>	E.00	C.1C	20		22		40		E . 00	E.1E	200		242		E30	
5:30 - 5:45 36 39 75 5:30 - 5:45 266 218 484 5:45 - 6:00 46 132 48 133 94 265 5:45 - 6:00 287 1103 264 930 551 2033 6:00 - 6:15 - 6:30 54 73 127 6:15 - 6:30 274 1187 461 6:30 - 6:45 - 58 88 146 6:30 - 6:45 210 220 430 6:645 - 7:00 83 239 102 316 185 555 6:45 - 7:00 232 976 187 867 419 1843 7:00 - 7:15 104 130 234 7:00 - 7:15 237 202 439 7:15 - 7:30 102 150 252 7:15 - 7:30 223 190 413 7:30 - 7:45 112 180 292 7:30 - 7:45 212 199 411 7:30 - 8:45 8:00 160 478 178 638 338 1116<																
5:45 - 6:00 46 132 48 133 94 265 5:45 - 6:00 287 1103 264 930 551 2033 6:00 - 6:15 - 6:30 44 53 97 6:00 - 6:15 - 6:30 274 187 461 6:30 - 6:45 - 58 88 146 6:30 - 6:45 - 210 220 430 6:45 - 7:00 83 239 102 316 185 555 6:45 - 7:00 232 976 187 867 419 1843 7:00 - 7:15 - 7:30 102 150 252 7:15 - 7:30 223 190 413 7:35 - 7:30 102 150 252 7:15 - 7:30 223 190 413 7:35 - 7:30 112 180 292 7:30 - 7:45 212 199 411 7:35 - 8:00 160 478 178 638 338 1116 7:45 - 8:00 198 870 178 769 376 1639 8:00 - 8:15 - 154 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>																
6:00 - 6:15				122		122		265				1103		ดวก		2033
6:15 - 6:30	0.40	0.00	40	102	70	100	J-T	200	0.40	0.00	LUI	1100	201	500	001	2000
6:15 - 6:30	6:00 -	6:15	44		53		97		6:00	- 6:15	260		273		533	
6:30 - 6:45																
6:45 - 7:00 83 239 102 316 185 555 6:45 - 7:00 232 976 187 867 419 1843 7:00 - 7:15 104																
7:15 - 7:30				239		316		555				976	187	867	419	1843
7:15 - 7:30																
7:30 - 7:45	7:00 -	7:15	104		130		234		7:00	- 7:15	237		202		439	
7:45 - 8:00	7:15 -	7:30	102		150		252		7:15	- 7:30	223		190		413	
8:00 - 8:15	7:30 -	7:45	112		180		292		7:30 -	- 7:45	212					
8:15 - 8:30	7:45 -	8:00	160	478	178	638	338	1116	7:45	- 8:00	198	870	178	769	376	1639
8:15 - 8:30					404		2.40		0.00	0.15			150		000	
8:30 - 8:45																
8:45 - 9:00																
9:00 - 9:15				704		000		1617				F70		F04		1160
9:15 - 9:30	8:45 -	9:00	192	124	238	893	430	1017	8:45	- 9:00	140	5/8	150	584	290	1102
9:15 - 9:30	0.00	0.15	10/		21/		200		0.00	0.15	121		124		255	
9:30 - 9:45																
9:45 - 10:00																
10:00 - 10:15				710		757		1467				504		527		1031
10:15 - 10:30	3.10	10.00	200	, 10	130	, , ,	101	1107	30	10.00			10	02.	20,	
10:15 - 10:30	10:00 - 1	10:15	202		164		366		10:00	- 10:15	108		102		210	
10:45 - 11:00	10:15 - 1	10:30	220		156		376		10:15 -	- 10:30	108		111		219	
11:00 - 11:15	10:30 - 1	10:45	196		177		373		10:30 -	- 10:45	81		96		177	
11:15 - 11:30	10:45 - 1	11:00	210	828		659	372	1487	10:45 -	- 11:00	85	382	84	393	169	775
11:15 - 11:30																
11:30 - 11:45																
11:45 - 12:00																

TOTALS 4,457 4,431 8,888 9,618 9,045 18,663 ADT'S 14,075 13,476 27,551																
ADT'S 14,075 13,476 27,551		******							*******	******						
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	2'TGA										1.	4 በ75	1	3 476	2	7 551
		*****	*****	*****	*****	*****	*****	*****	******	******						

LOCATION - NEWPORT-BTN 32ND/31ST

AVERAGED VOLUMES FOR - TUESDAY 6/3/08 TO WEDNESDAY 6/4/08

TIME	N.		SI		TOT			IME	N			B		TAL
*********** 00 - 12:15	44	*****	******* 46	*****	90	*****		- 12:15	210	*****	276	****	486	
15 - 12:30	54		35		89		12:15		202		286		488	
30 - 12:45	39		38		77		12:30		222		272		494	
45 - 1:00	37	174	25	144	62	318	12:45		240	874	290	1124	530	19
00 - 1:15	40		22		62		1:00	- 1:15	212		238		450	
15 - 1:30	30		15		45		1:15		266		242		508	
30 - 1:45	33		23		56			- 1:45	202		240		442	
45 - 2:00	22	125	16	76	38	201	1:45	- 2:00	224	904	256	976	480	1
00 - 2:15	24		18		42		2:00 -	- 2:15	208		232		440	
15 - 2:30	12		11		23		2:15 -	- 2:30	226		255		481	
30 - 2:45	0		4		4		2:30 -	- 2:45	226		216		442	
45 - 3:00	11	47	9	42	20	89	2:45	- 3:00	240	900	255	958	495	1
00 - 3:15	11		5		16		3:00	- 3:15	227		237		464	
15 - 3:30	8		6		14		3:15 -	- 3:30	196		245		441	
30 - 3:45	6		3		9		3:30 -	- 3:45	232		270		502	
45 - 4:00	5	30	5	19	10	49	3:45	4:00	236	891	244	996	480	1
00 - 4:15	6		4		10		4:00 -		241		238		479	
15 - 4:30	8		2		10		4:15 -		229		256		485	
30 - 4:45	9		12		21		4:30 -		216		264		480	
45 - 5:00	14	37	11	29	25	66	4:45 -	- 5:00	212	898	290	1048	502	1
00 - 5:15	24		14		38		5:00 -		228		300		528	
15 - 5:30	24		28		52		5:15 -		244		306		550	
30 - 5:45	29		36		65		5:30 -		236		329		565	_
45 - 6:00	40	117	44	122	84	239	5:45 -	- 6:00	191	899	342	1277	533	2
00 - 6:15	77		62		139		6:00 -		215		341		556	
15 - 6:30	88		82		170		6:15 -		200		344		544	
30 - 6:45	109		98		207		6:30 -		236		320		556	_
45 - 7:00	148	422	118	360	266	782	6:45 -	7:00	214	865	312	1317	526	2
00 - 7:15	182		150		332		7:00 -		221		306		527	
15 - 7:30	171		144		315		7:15 -		224		234		458	
30 - 7:45	247		156		403		7:30 -		208		249		457	_
45 - 8:00	240	840	203	653	443	1493	7:45 -	- 8:00	200	853	192	981	392	1
00 - 8:15	235		186		421			8:15	212		192		404	
15 - 8:30	232		174		406			8:30	180		198		378	
30 - 8:45 45 - 9:00	223 197	887	179 190	729	402 387	1616	8:30 - 8:45 -	8:45 9:00	202 204	798	195 170	755	397 374	1
				•										
00 - 9:15	185		179		364 276			9:15	188		144		332 376	
15 - 9:30	174		202		376 397			9:30 9:45	220 136		156 119		255	
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'S									17	1,323	1.	5,321	2	9.

LOCATION - BALBOA-BTN 19TH/18TH

AVERAGED VOLUMES FOR - TUESDAY 6/3/08 TO WEDNESDAY 6/4/08

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**************************************	FB	WB	TOTAL	TIME	EB	WB	TOTAL
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12:00 - 12:15	37	26	63	12:00 - 12:15	194	175 158	369 338
12:15 - 12:30	28	26 22	54 54	12:15 - 12:30 12:30 - 12:45	180 174	152	326
12:30 - 12:45 12:45 - 1:00	32 23 120	19 93	42 213	12:45 - 1:00	242 790	196 681	438 1471
1.00 1.15	1.0	10	37	1:00 - 1:15	151	138	289
1:00 - 1:15 1:15 - 1:30	18 19	19 16	35	1:15 - 1:30	190	216	406
1:30 - 1:45	21	16	37	1:30 - 1:45	188	156	344
1:45 - 2:00	13 71	9 60	22 131	1:45 - 2:00	198 727	172 682	370 1409
2:00 - 2:15	12	10	22	2:00 - 2:15	184	163	347
2:15 - 2:30	12	14	26	2:15 - 2:30	189	182	371
2:30 - 2:45	5	5	10	2:30 - 2:45	223	192	415 405 1538
2:45 - 3:00	9 38	5 34	14 72	2:45 - 3:00	199 795	206 743	400 1000
3:00 - 3:15	3	5	8	3:00 - 3:15	193	163	356
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6:30 - 6:45 6:45 - 7:00	58 81 223	83 132 362	141 213 585	6:45 - 7:00	226 1040	168 687	394 1727
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7:00 - 7:15 7:15 - 7:30	106	156 162	262 284	7:00 - 7:15 7:15 - 7:30	236 208	177	385
7:15 - 7:30 7:30 - 7:45	122 108	207	315	7:30 - 7:45	180	148	328
7:45 - 8:00	224 560	257 782	481 1342	7:45 - 8:00	163 787	148 632	311 1419
8:00 - 8:15	179	254	433	8:00 - 8:15	146	148	294
8:15 - 8:30	114	200	314	8:15 - 8:30	140	144	284
8:30 - 8:45	117	226	343	8:30 - 8:45	142	134	276
8:45 - 9:00	140 550	166 846	306 1396	8:45 - 9:00	130 558	137 563	267 1121
9:00 - 9:15	138	154	292	9:00 - 9:15	118	116	234
9:15 - 9:30	138	170	308	9:15 - 9:30	135	169	304
9:30 - 9:45	153	159	312	9:30 - 9:45	92	78 74 437	170 155 863
9:45 - 10:00	161 590	171 654	332 1244	9:45 - 10:00	81 426	74 437	100 800
10:00 - 10:15	160	148	308	10:00 - 10:15	92	100	192
10:15 - 10:30	132	154	286	10:15 - 10:30	93	66 60	159 130
10:30 - 10:45	132	162	294 354 1242	10:30 - 10:45 10:45 - 11:00	62 61 308	68 52 286	113 594
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TOTALS	3,539	4,336	7,875		8,192	7,107	15,299
ADT'S					11,731	11,443	23,174
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APPENDIX E

DEFINITIONS

Certain terms used throughout this report are defined below to clarify their intended meaning:

ADT Average Daily Traffic. Generally used to measure the total two-directional

traffic volumes passing a given point on a roadway.

DU Dwelling Unit. Used in quantifying residential land use.

ICU Intersection Capacity Utilization. A measure of the volume to capacity ratio

for an intersection. Typically used to determine the peak hour level of service

for a given set of intersection volumes.

LOS Level of Service. A scale used to evaluate circulation system performance

based on intersection ICU values or volume/capacity ratios of arterial

segments.

Peak Hour This refers to the hour during the AM peak period (typically 7 AM - 9 AM) or

the PM peak period (typically 3 PM - 6 PM) in which the greatest number of vehicle trips are generated by a given land use or are traveling on a given

roadway.

TSF Thousand Square Feet. Used in quantifying non-residential land uses, and

refers to building floor area.

V/C Volume to Capacity Ratio. This is typically used to describe the percentage of

capacity utilized by existing or projected traffic on a segment of an arterial or

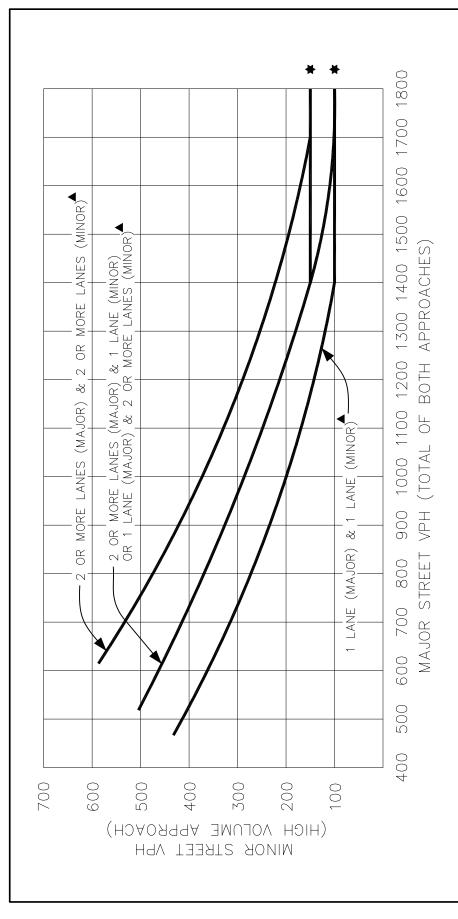
intersection.

VPH Vehicles Per Hour. Used for roadway volumes (counts or forecasts) and trip

generation estimates. Measures the number of vehicles in a one hour period,

typically the AM or PM peak hour.

APPENDIX F PEAK HOUR SIGNAL WARRANT



NOTE:

THESE CURVES ARE RECOMMENDED FOR USE IN AREAS OF URBAN CLASSIFICATION (i.e. POSTED SPEED LIMIT ON THE MAJOR STREET IS 64 km/hr or 35 MPH OR LESS). 150 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES, AND 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE. NOTE: ¥

Source: MUTCD - Figure 4C-3

Figure F-1

PEAK HOUR SIGNAL WARRANT



Walker Parking Consultants 2550 Hollywood Way, Suite 303 Burbank, CA 91505

Voice: 818.953.9130 Fax: 818.953.9331 www.walkerparking.com

October 24, 2008

Anthony Brine, P.E., Principal Civil Engineer City of Newport Beach 3300 Newport Boulevard Newport Beach, CA 92663

Re: Parking Management Recommendations – Marina Park Project (revised)
Newport Beach, California

Dear Mr. Brine,

Walker Parking Consultants is pleased to present this letter report covering the parking management alternatives for the Marina Park project on Balboa Peninsula.

SUMMARY

In discussing parking management issues with other community centers and sailing facilities in Southern California we have determined that parking management issues at similar facilities present unique and distinct challenges. We also determined that parking capacity issues at Marina Park are likely to occur during peak summer weekends, indicating that a permanent, year-round solution may not be the most efficient solution for this area. Rather, flexible solutions that can be modified and managed rapidly may be the best way to ensure efficient and available parking. From discussions with City staff, we understand that enforcement beyond 8:00 AM to 6:00 PM may be difficult to attain. Nonetheless, our recommendations sometimes include extending the hours of enforcement, when appropriate, for efficient operation. With this in mind, we recommend that the City develop the following initiatives to help manage parking at Marina Park:

- 1. Charge a fee for parking to help turn over the spaces and ensure the lot is available for beach goers and Marina Park patrons.
- 2. Parking fees should be enforced 24 hours per day to help manage overnight parking by residents that may otherwise interfere with access to the Marina Park community center.
- 3. Install multi-space meters at the lot to enable automatic payment for all spaces in all lots to efficiently manage the payment system.
- 4. Install flip signs to quickly, inexpensively and efficiently reserve spaces during events at the community center, Girl Scout House, marina, and sailing center.

In general, parking in Balboa Peninsula is impacted during the summer weekends from June to September. The peak demand period begins in the early afternoon and does not subside until nearly sunset. If events at Marina Park do not coincide with these peak demand periods, parking can be managed with simple economic cues. During peak summer periods, events should be scheduled either early in the morning (e.g. beginning at 8:00AM) or later in the evening to avoid the impact from the beach visitors.



PROJECT DESCRIPTION

The City of Newport Beach is creating a new plan for Marina Park that includes a community center, sailing center, and marina. The site is located between 15th Street and 18th Street along the bay side of the Newport Peninsula and currently includes various recreational uses and approximately 60 mobile homes. The proposed plan for the site consists of a10,200 square foot community center, an 11,200 square foot sailing center and café, a recreational park, and a 28-berth visitor marina. The Girl Scout House will also be relocated from its current location to the northwest corner of the project site. The recreational amenities at the park will include a playground, tennis courts, basketball courts, beach volleyball courts, and open park areas. In addition, docks and slips for sailing programs and improved beach access will be provided. Figure 1 outlines the proposed location of the Marina Park development.

Figure 1: Project Study Area



Source: Google Earth Pro, Accessed September 24, 2008.

Figure 2 shows the proposed project site plan and related parking areas.

Figure 2: Project Site Plan



Source: City of Newport Beach, September 2008.





PROJECT BACKGROUND

Access to Marina Park will be critical to the success of the development. A parking study examining the myriad uses was conducted by Austin-Foust in July 2008. Our understanding is that the City is satisfied with the projections from the Austin-Foust report; therefore, we have not studied the parking generation for this project.

Our understanding is that parking at the Marina Park project is being developed to ensure that patrons and visitors using the facilities at Marina Park have appropriate parking and access to those facilities. During the summer months, parking will likely be problematic because as the Newport Peninsula beach lots and nearby on-street parking becomes unavailable many beach goers will likely park in any nearby space. With over 150 spaces in the main lot and the nearby "Girl Scout" lot, the Marina Park complex (Marina Park) will likely be used by beach visitors or other excursionists on the central part of the Peninsula. If the price for parking at Marina Park is attractive, beach goers and nearby residents will immediately fill the lot, leaving the Marina Park users unable to park anywhere near their intended destination. If beach goers and residents have a greater willingness to pay than Marina Park patrons then the Marina Park parking lot will be unavailable to Marina Park patrons throughout much of the summer, and particularly during the weekends. Our belief is that demand for parking on Newport Peninsula is nearly unlimited during the busy summer season. If supply is continually developed and subsidized, effectively removing economic considerations, parking will continue to be a problem throughout Newport Beach and near the Marina Park area specifically.

PARKING MANAGEMENT

In our earlier discussion and analyses we referred to many of the elements of parking management. Parking Management includes myriad strategies aimed at making better use of the available parking supply in any defined area. Proper parking management incorporates a number of goals, but a core principle is that parking spaces should be used efficiently. Parking spaces that sit unoccupied are inefficient as they represent significant financial and land resources, as well as the opportunity costs of the funds and real estate, that is not devoted to productive uses. This is especially true in a desirable place such as Newport Beach. It is also true where competition for impacted parking spaces exists a short distance away. The time and frustration that results from the search for a convenient space in these impacted areas represents inefficiencies.

Parking management practices attempt to address inefficiencies through restrictions and parking pricing. We aim to allocate parking spaces for which there is high demand with user restrictions and/or prices to park. For spaces for which there is low demand, we relax parking restrictions and lower prices in order to maximize their utilization. In high demand area we recommend increased prices, strict enforcement, and greater turnover of the parking spaces.

If spaces in high demand are free and/or spaces in low demand are priced, inefficiencies are created. We note that parking spaces are a finite resource and represent a real cost. We wish to allocate that resource as efficiently as possible. Finally, although not always politically popular, pricing is virtually always the most effective way to manage parking spaces.



ALTERNATIVES

This report is focused on managing the future parking at the Marina Park development. In an effort to conform to the likely requirements of the California Coastal Commission, our recommendations try to support equal access to all visitors and patrons of Marina Park as well as the beach going public. This is particularly important during the summer months from June to September. As we have observed from other parking studies conducted in the area, parking during weekend days throughout the summer will be nearly impossible to manage due to the significant demand and presumed willingness of infrequent beach goers to pay for parking during their excursion to the Newport beaches. To mitigate demand and help manage the parking, we have come up with several alternatives for the City to consider. The following is a list outlining the possible parking management strategies to help prioritize parking at Marina Park.

1. Install automated multi-space meters. In an area that has virtually unlimited demand for parking and a commensurate shortage of land, one of the few solutions to help manage parking is to allocate the spaces to those who are most willing to pay. That is, we recommend installing parking meters. This may seem inherently unfair, or regressive; however, it helps alleviate parking in the less expensive, less desirable areas by removing vehicles that are willing to pay a premium for a space from the on-street spaces they would otherwise occupy. Given the size and circulation of the parking lots at Marina Park, we believe that multi-space pay-and-display meters will be the best solution, used in conjunction with any other combination of parking



management that we outline in this report. To ensure optimal efficiency, pricing in the lot should be marginally higher than nearby on-street parking meter fees.

- 2. Charge for overnight parking. To help ensure that residents or businesses do not over utilize the Marina Park parking lot, we recommend that a paid parking system be in operation at all times. This will ensure that there are no cars occupying the lot that are not there for a specific event or purpose. This will help ensure the lot does not become a long-term storage area for vehicles. If the lot is open, and free to residents overnight, having the lot available for morning classes or sailing lessons will be difficult to achieve.
- 3. Do not accept master park permits. Another measure that should be included in the plan is that the Marina Park lot should not accept Newport Beach master park permits, blue pole permits or any other exempt parking permit that is accepted in other areas of the City. Again, this will help ensure that the Marina Park lot is available for Marina Park patrons, visitors and guests.

These first three recommendations will help ensure that the lot is primarily utilized by day visitors to Marina Park and not by long-term excursionists to other destinations or by residents seeking inexpensive and convenient parking. The alternatives below may help supplements these solutions by segmenting parking at Marina Park. It should be noted that these alternatives are more nuanced than the first three recommendations and have additional limitations that should be considered by the City.



BEST OPTIONS—THOUGH NOT LIKELY TO BE APPROVED BY COASTAL COMMISSION

- 4. Marina Park Only Parking. The most obvious solution to achieve the City's goal of ensuring the Marina Park lot is only used by its intended patrons is to restrict Marina Park parking to only the patrons, visitors and guests of the Marina, Sailing Center, Girl Scout Center, Community Center or Park. Of course this would be extremely difficult to enforce because it would not be readily apparent which vehicles were using the Marina Park facilities and which vehicles where using the beach or other nearby land uses. Permits could be used for community center visitors, or other registered users, but issuing permits for park users would be difficult to identify and control. In addition, we do not feel that this solution would be agreeable with the Coastal Commission's stated goal of ensuring equal access to ALL beach visitors.
- 5. Validation. If there is a pricing mechanism for the Marina Park lot, all patrons, visitors, users, and guests could receive a validation sticker to off-set or eliminate their cost to park in the lot. If pricing at the Marina Park lots is significantly higher than the surrounding spaces, it is likely to discourage all but the most price-insensitive patrons to park elsewhere. Of course, this again means that the lot will be underutilized except when there is an event at Marina Park. From our experience it is unlikely that the Coastal Commission will agree to this sort of validation system since it creates a preferential parking system that limits or restricts public access to the nearby beaches.
- 6. Reverse Validation. If the price to park at the Marina Park lot was twice as high as nearby onstreet or surface lot users of Marina Park facilities could be offered a rebate on their parking fees for whichever activity they are pursuing at Marina Park (reverse validation). Rather than validate parking (offsetting the price of parking) the user could pay the same price for parking as other users (beach visitors, residents and others seeking parking) but bring their parking receipt into the Marina Park offices to receive a discount on their sailing lesson, community center activity, or moorage fee. This system could be a bit confusing to infrequent users and may not be readily accepted by many Marina Park patrons, visitors, or guests. While technically everyone would pay the same fee for parking, the Coastal Commission may not approve such a minor technicality. As a result of these uncertainties, this may not be the most desirable solution.

OTHER OPTIONS - NOT RECOMMENDED

7. Pricing. If the Marina Park lot is priced significantly higher than nearby beach lots and on-street spaces it is likely that the lot will fill up only after all other areas spaces fill. This strategy may help ensure that Marina Park has as much availability as possible for as much time as possible. One drawback to this strategy is that during peak demand periods this lot will eventually fill up and Marina Park patrons will not have access. Another problem is that the lot will be significantly underutilized throughout the remainder of the year, as neither beach visitors, residents, nor Marina Park patrons will want to park in the most expensive spaces except during high demand days or during busy events. A market-rate pricing scheme that adjusts prices based on demand (occupancy) is recommended.



8. Create priority areas for the Marina Park users. Another option is to create a parking system that sets aside (either with nesting gates or chains) a certain number of spaces for visitors that have Marina Park reservations for daily activities. As classes, events, or other reservations are made for the Marina Park users, parking could be paid in advance, along with their other fees, this helps ensure the Marina Park user that they will have a parking space when they come to Marina Park. This option would be fairly labor intensive because it would likely require that spaces be set aside in advance of any classes or events at the center and would essentially reduce the total number of available parking spaces for much of the day. A parking attendant would be recommended to ensure that parking is appropriately assigned. This sort of inefficiency is not desirable from a parking management perspective and would not likely be approved by the Coastal Commission.

RECOMMENDED OPTIONS

9. Install flip signs. Rather than create "Marina Park Parking Only" areas, a sign that changes dependent upon the expected amount of daily visitors can be installed. We often call these "flip signs" since they can flip open to display a message or flip closed to display a different message (or no message). Flip signs can be installed in some or all of the spaces (including near the Girl Scout Center) at Marina Park. When the facility has an event during the peak season, the signs can display a "Reserved For Marina Park Visitors Only" (or "Reserved for Girl Scout House") sign. When there is no



Source: emedco.com

event, the sign can be flipped closed; opening the space to all visitors or guests to the area. Event reservations can be used to help determine the appropriate number of spaces to reserve.

10. Increase bicycle/pedestrian facilities. Demand for parking can be reduced by providing bicycle and pedestrian facilities and amenities that make it easier and more pleasant to bicycle or walk to nearby destinations. This strategy could prove to be particularly valuable for a community center in an area like Balboa Peninsula. Some cities have also begun experimenting with bike stations or full-service bike lockers near destinations that provide lockers, changing rooms and showers for bicycle commuters. Bikestation services include secure, indoor bicycle parking available to members with a membership pass. In the Seattle Bikestation there is free attended bicycle parking during operating hours. Bicycle repair services and commuter retail items are also available at this facility, as well as public transportation schedules, bike maps, and a personalized service matching new bicycle commuters with experienced cyclists who can help them plan a commute route, provide tips on bicycle commuting, and generally serve as mentors.¹

Bikestations are currently in use in several California cities including Long Beach, Palo Alto, and San Francisco.

¹ Metro King County Government, http://transit.metrokc.gov/tops/bike/bikestation.html, accessed September 29, 2008.





RECOMMENDATIONS

The goal of this report is to provide the City with parking management solutions that will likely help provide the appropriate amount of access to Marina Park without restricting beach goers from parking at the area lots. We believe that a combination of solutions will be the most effective approach to achieving this goal. Namely, we believe that paid parking will be essential to help manage this facility. A multi-space, pay-and-display system that operates 24-hours per day will likely be the most efficient solution to help manage the parking. In addition, a fee that is slightly higher than the nearby on-street spaces will likely ensure that the Marina Park lots are the last to be occupied during busy summer peak Flip signs that alternate between "Reserved For Marina Park Visitors Only" and demand periods. "Parking Available" should be installed to help manage parking during events at Marina Park. To accomplish this, the Marina Park lots can estimate the number of users for any event, and flip open the "Reserved Parking" side of the sign to help ensure that Marina Park patrons have access to the facility. At all other times, the signs can be flipped closed, or display a "Parking Available" message to enable all vehicles on the Peninsula to use the parking facility. Finally, while not necessarily a parking management technique, demand for parking in general could be reduced at Marina Park by providing ample bicycle and pedestrian access and facilities.

If you have any questions regarding our report, please do not hesitate to contact us.

Sincerely,

Mark Linsenmayer

WALKER PARKING CONSULTANTS

cc: Steffen Turoff Rosalinh Ung